GENERAL NOTES

FOUNDATIONS

SOIL UNDER SLABS AND UNDER FOOTINGS TO BE COMPACTED TO AT LEAST 95% OF MAX. DRY DENSITY AS DETERMINED BY ASTM D1557 (MODIFIED PROCTOR) MINIMUM BEARING CAPACITY 2000 PSF

TERMITE TREATMENT

REFER TO SECTION R318 PROTECTION AGAINST TERMITES, OF THE FLORIDA BUILDING CODE - RESIDENTIAL CURRENT EDITION, FOR FULL LIST OF REQUIREMENTS

CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI, A SLUMP OF 4" PLUS OR MINUS 1", AND HAVE 2 TO 4% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.58.
2. ALL REINFORCING STEEL SHALL BE DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60 MIN.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-I85 FOR 6x6xW1.4xW1.4 WWF SHALL BE LAPPED AT LEAST 6" & CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6".FIBERMIX OF EQUAL SPECIFICATIONS MAY BE USED IN LIEU OF WWF.
4. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318 LATEST EDITION, AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS," ACI 301.
5. HORIZONTAL FOOTING BARS SHALL BE BENT MIN. 12 BAR DIAMETERS (EXCLUDING BEND) AROUND CORNERS OR CORNER BARS WITH MIN. 24" LAP EXCLUDING BEND AT EACH END SHALL BE PROVIDED.
6. MINIMUM LAP SPLICES ON ALL REINFORCING BARS SHALL BE PER DETAIL MD08.
7. SLAB ON GRADE, REINFORCEMENT SHALL BE SUPPORTED IN PLACE FROM THE CENTER TO UPPER 1/3 OF THE SLAB.

BOLTS AND THREADED RODS

1. ALL BOLTS & THREADED RODS TO BE ASTM A307 OR BETTER (U.N.O.)
2. STEEL BOLTS LESS THAN ONE-HALF-INCH (12.7 mm) IN DIAMETER USED AS FASTENERS FOR PRESSURE PRESERVATIVE-TREATED WOOD SHALL BE OF HOT-DIPPED ZINC COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. FASTENERS SHALL BE IN ACCORDANCE WITH ASTM A 153.
3. FASTENERS OTHER THAN NAILS AND TIMBER RIVETS SHALL BE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC-COATED STEEL WITH COATING WEIGHTS IN ACCORDANCE WITH ASTM B 695, CLASS 55, MINIMUM.

MASONRY WALL CONSTRUCTION

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, W/ A MINIMUM NET COMPRESSIVE STRENGTH OF 2000 PSI (f'm = 1900 PSI)
2. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270
3. HEAD MORTAR JOINTS AT PRECAST WINDOW SILLS TO BE NO MORE THAN 1".
4. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.
5. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH CELLS FILLED WITH COARSE GROUT AND GRADE 60 MIN. STEEL. REINFORCEMENT SHALL BE PLACED IN CENTER OF MASONRY CELL W/ MIN 1/2" CLEARANCE TO INSIDE FACE.
6. VERTICAL REINFORCEMENT SHALL BE HELD IN CENTER OF THE MASONRY/ BLOCK WALL AT THE TOP & BOTTOM & MAXIMUM 8'-0" U.N.O. VERTICALLY. TIES TO THE FOOTING & LINTEL STEEL IS ADEQUATE @ TOP & BOTTOM OF WALL; REBAR POSITIONERS IN MIDDLE OF WALL
7. REINFORCING STEEL SHALL BE LAPPED AND HOOKED AT THE TOP AND BOTTOM TO THE BOND COURSE AND FOOTING PER MD08. NO NON-STRUCTURAL ITEMS (SUCH AS PLUMBING) SHALL BE PLACED WITHIN FILLED CELLS
8. ALL BUNDLED BAR REINFORCEMENT SHALL BE TIED TOGETHER W/ MIN. 16 GA. TIES SUCH THAT DISPLACEMENT WILL NOT OCCUR DURING THE CONRETE PLACEMENT OPERATION
9. EXPANSION TYPE ANCHORS ARE NOT TO BE USED IN BOND COURSE. EMBEDDED ANCHORS OR EPOXY FASTENED STUDS SHALL BE USED.
10. MASONRY ROUGH OPENING TOLERANCES - 1/4" TO + 1/2"
11. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW OF GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED.
12. TEMPORARY BRACING AND SHORING OF WALL TO PROVIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR
13. DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS AND NO CONCENTRATED LOADS FOR (7) DAYS. PER CODE ACI 318, LATEST EDITION.
14. DURING CONCRETE POURS, GC TO ADEQUATELY VIBRATE THE FILLED CELL W/ EITHER RODDING OR PENCIL VIBRATOR TO ENSURE PROPER CONCRETE CONSOLIDATION.
15. VERTICAL FURRING STRIPS WHEN REQUIRED TO BE MAX. 2x P.T. ATTACHED TO CMU W/ MIN. 0.131 DIAMETER CASE HARD MASONRY NAIL W/ MIN. 1 1/4" EMBEDMENT OR 1/4" TAPCON W/ MIN. 1 1/2" EMBEDMENT @ 16" O.C. MEASURED VERTICALLY AND 4" FROM ENDS. 2x FURRING TO BE INSTALLED PER MANUFACTURER'S WRITTEN SPECIFICATIONS OR @ MAX 24" O.C.

WOOD CONSTRUCTION

1. WOOD CONSTRUCTION SHALL CONFORM TO THE ANSI/AWC "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION.(NDS)
2. LUMBER DENOTED AS SPF#2 OR SPF#1/#2 SHALL BE SPRUCE-PINE-FIR VISUALLY GRADED "No.1/No.2" BY NLGA, OR BETTER.
3. LUMBER DENOTED AS SYP#2 SHALL BE SOUTHERN YELLOW PINE OR "MIXED" SOUTHERN YELLOW PINE VISUALLY GRADED "No 2" BY SPIB, OR BETTER.
4. OTHER LUMBER SHALL BE VISUALLY GRADED "No. 2" OR BETTER, REGARDLESS OF SPECIES, U.N.O. ON FRAMING PLANS OR DETAILS.
5. ALL STRUCTURAL WOOD MEMBERS INCLUDING EXTERIOR FRAME WALLS, BEARING WALLS, SHEAR WALLS, AND MISC. MEMBERS (I.E. BLOCKING OR GABLE END BRACING) SHALL BE SPF#2 OR BETTER U.N.O..
6. END-JOINTED LUMBER IS NOT PERMITTED TO BE USED IN ANY LOAD BEARING (GRAVITY OR WIND) CONDITIONS. E.O.R. TO REVIEW IF INSTALLED.
7. FIELD-CUT ENDS, NOTCHES AND DRILLED HOLES OF PRESERVATIVE-TREATED WOOD SHALL BE TREATED IN THE FIELD IN ACCORDANCE WITH AWPA M4.
8. THE PORTIONS OF GLUED-LAMINATED TIMBERS THAT FORM THE STRUCTURAL SUPPORTS OF A BUILDING OR OTHER STRUCTURE AND ARE EXPOSED TO WEATHER AND NOT PROPERLY PROTECTED BY A ROOF, EAVE OR SIMILAR COVERING SHALL BE PRESSURE TREATED WITH PRESERVATIVE, OR BE MANUFACTURED FROM NATURALLY DURABLE OR PRESERVATIVE-TREATED WOOD.
9. ALL LUMBER SPECIFIED ON DRAWINGS IS INTENDED FOR DRY USE ONLY (MOISTURE CONTENT 19% OR LESS), U.N.O. ALL WATERPROOFING AND FIRE SAFETY SYSTEMS ARE THE RESPONSIBILITY OF THE CONTRACTOR AND ARE TO BE DESIGNED AND DETAILED BY OTHERS.
10. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION SHIELDS. ALL HOLES OVER 1" IN DIA FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP U.N.O.
11. MANY OF THE PRESSURE TREATED WOODS USE CHEMICALS THAT ARE CORROSIVE TO STEEL. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TYPE OF WOOD TREATMENT AND TO SELECT APPROPRIATE CONNECTORS THAT RESIST CORROSION. FOR EXAMPLE. ACQ-C, ACQ-D, CBA-A OR CA-B REQUIRE HOT-DIPPED GALVANIZED OR STAINLESS STEEL FASTENERS. DOR SODIUM BORATE (SBX) DOES NOT.
12. ALL EXPOSED WOOD OR WOOD IN CONTACT WITH EARTH, CONCRETE OR MASONRY IS TO BE PRESSURE TREATED IN ACCORDANCE WITH THE LATEST EDITION OF THE AWPA U1 STANDARD.
13. UNTREATED WOOD SHALL NOT BE IN DIRECT CONTACT WITH CONCRETE OR MASONRY. SEAT PLATES SHALL BE PROVIDED AT BEARING LOCATIONS WITHOUT WOODEN TOP PLATES.
14. SEE PLAN FOR BUILT UP STUD COLUMN AND BEAM NAILING PATTERNS

ENGINEERED LUMBER

1. ENGINEERED LUMBER PRODUCTS SHALL CONFORM TO THE CURRENT EDITION OF THE ANSI/AWC NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS).
2. ENGINEERED LUMBER SHALL BE EITHER STRUCTURAL COMPOSITE LUMBER AS DEFINED NDS SECTION 8.1 (INCLUDING LVL, PSL, AND LSL LUMBER)
3. ALL ENGINEERED LUMBER IS TO HAVE THE MINIMUM DESIGN VALUES AS FOLLOWS:

COLUMNS	BEAMS	STUD
E = 1,700,000 lb/in^2 U.N.O.	E = 2,000,000 lb/in^2 U.N.O.	E = 1,500,000 lb/in^2 U.N.O.
Fb = 2650	Fb = 3000	Fb = 2200
Ft = 1650	Ft = 1700	Ft = 1500
Fc $\parallel$= 3000	Fc $\parallel$= 3000	Fc $\parallel$= 2100
Fc $\perp$ = 450	Fc $\perp$ = 900	Fc $\perp$ = 450
Fv = 285	Fv = 285	Fv = 150

WOOD BEARING SURFACES ARE TO BE SYP#2 OR BETTER END GRAIN CONDITION U.N.O.

CEILING CONSTRUCTION

1. USE 1/2" CEILING OR SAG RESISTANT BOARD FOR 24" O.C. FRAMING U.N.O.
2. 5/8" TYPE "X" GYPSUM BOARD SHALL BE USED ON CEILING IN GARAGE WHEN LIVING SPACE IS ABOVE THE GARAGE.

MEANS AND METHODS

CONSTRUCTION MEANS & METHODS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE STRUCTURE IS UNSTABLE UNTIL ALL LOAD BEARING WALLS ARE ERECTED & TRUSS / FRAME MEMBERS ARE CONNECTED PROPERLY PER APPROVED FRAMING PLANS / DETAILS ALONG W/ CONNECTOR & COMPONENT MANUFACTURERS' SPECIFICATIONS. UNTIL SUCH TIME, TEMPORARY BRACING & SHORING OF WALLS AND FRAME MEMBERS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

UNDER STAIR PROTECTION

1. ENCLOSED ACCESSIBLE SPACE UNDER STAIRS SHALL HAVE WALLS, UNDER STAIR SURFACE AND ANY SOFFITS PROTECTED ON THE ENCLOSED SIDE WITH 1/2-INCH (12.7MM) GYPSUM BOARD.

METAL COATINGS FOR USE WITH PRESSURE TREATED WOOD

IT IS THE BUILDER'S RESPONSIBILITY TO ENSURE THAT ALL METAL CONNECTORS, METAL FASTENERS, ANCHORS, AND NAILS ARE THE APPROPRIATE MATERIAL AND/OR HAVE THE APPROPRIATE COATING FOR USE WITH THE DIFFERENT TYPES OF PRESSURE TREATED WOODS, AS PER THE FOLLOWING GUIDELINES.

1. WOOD TREATED WITH DOT SODIUM BORATE(SBX): CONNECTORS SHALL BE A MINIMUM G50 ZINC COATING
2. WOOD TREATED WITH ACQ-C OR ACQ-D (CARBONATE) OR OTHER BORATE (NON-DOT) TREATMENTS: CONNECTORS SHALL BE A MINIMUM G185 ZINC COATING.
3. FOR ALL OTHER COMBINATIONS FOLLOW THE RECOMMENDATIONS OF THE PRESERVATIVE WOOD TREATER.
4. STAINLESS STEEL CONNECTORS AND FASTENERS MAYBE USED WITH ALL TYPES OF PRESSURE TREATED WOOD SUCH THAT THEY MEET THE REQUIREMENTS OF ASTM F1667

PREFABRICATED WOOD TRUSSES

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS.UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT OR LATERAL FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE COORDINATE WITH THE TRUSS ENGINEER FOR THE LOCATION OF THESE WALLS AND STRUCTURAL PLANS FOR MORE INFO.
2. TRUSSES SHALL BE DESIGNED BY MWFRS METHODOLOGY FOR LONG SPAN TRUSSES TO DETERMINE UPLIFT AND REACTION VALUES. MEMBER AND PLATE DESIGN TO BE CALCULATED BY COMPONENTS AND CLADDING METHOD UNLESS SPECIFIED OTHERWISE BY TRUSS ENGINEER OF RECORD
3. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF ANSI TPI 1.
4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY TRUSS MANUFACTURER UNLESS NOTED ON PLANS
5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY TRUSS MANUFACTURER IN ACCORDANCE WITH THE DESIGN LOADS.

TRUSS DESIGN LOADS :
TOP CHORD: Lr = 20 PSF
DL = 7 PSF (SHINGLE TYPE ROOF COVER)
DL = 15 PSF (LIGHT WEIGHT CLAY TILE ROOF COVER)
DL = 14 PSF (TRUSS OVERBUILD)
BOTTOM CHORD: DL = 10 PSF
MINIMUM FLOOR DESIGN LIVE LOADS:
A) FLOORS = 40 PSF B) BALCONIES = 60 PSF / 100 PSF (IF BALCONY > 100 SQ FT)
C) DECKS = 40 PSF D) STAIRS = 40 PSF
E) BONUS ROOM/GAME ROOMS = 60 PSF U.N.O.
MINIMUM FLOOR DESIGN DEAD LOADS:
A) FLOORS = 15 PSF B)BALCONIES = 15 PSF
GENERAL LOADING:
ATTIC WITHOUT STORAGE = 10 PSF
ATTIC WITH LIMITED STORAGE = 20 PSF
GUARDRAILS / HANDRAILS = 200 LB POINT LOAD / 50 PSF
GUARDRAILS IN-FILL COMPONENTS = 50 PSF
GENERAL TRUSS BRACING & BLOCKING REQUIREMENTS

-ADDIT'L BRACING & BLOCKING PER BCSI-1 & TRUSS MANUFACTURER'S DWGS

-G.C. TO PROVIDE MIN 2x SYP #2 BLOCKING BETWEEN TRUSSES TO COMPLY W/ APA RECOMMENDATIONS FOR MIN. 7/16" OSB SHEATHING

-MIN 2x SYP #2 BLOCKING REQ'D ALONGSIDE TRUSS TOP CHORD @ ROOF HIP/RIDGE TO PREVENT SHEATHING SPAN FROM EXCEEDING 2FT (NOT REQ'D IF SHEATHING SPAN IS LESS THAN 2FT). BLOCKING TO TRUSS TOP CHORD w/ (2) 12d NAILS @ 12" O.C. MAX & 4" FROM EA. END. NOTE: PLYWOOD EDGES @ HIP CORNERS REQUIRE MIN 2x4 SYP #2 BLOCKING w/ (3) 12d TOENAILS EACH END.

-TRUSS "X" BRACING: MIN 2x4 SYP #2 AT ANGLE APPROX. 45° FROM VERT. TO EA. TRUSS MEMBER w/ (3) 12d NAILS. SPACE "X" BRACING ALONG TRUSS SPAN @ 10'-0" MAX AND EQUALLY SPACED FROM END OF TRUSS. "X" BRACING TO START 20'-0" MAX FROM #1 HIP GIRDER AND / OR (4) BAYS AFTER GABLE END BRACING. SEE BCSI-B3 SECTIONS 1, 2 AND 3 FOR ADDITIONAL ROOF PLANE PERMANENT BRACING REQUIREMENTS.

-PERMANENT 2x4 SPF #2 (MIN.) CONT. LATERAL TRUSS BOTTOM CHORD BRACING PERPENDICULAR TO TRUSS SPAN AT 15'-0" O.C. MAX W/IN TRUSS SPAN OR PER TRUSS DESIGN DRAWINGS. ATTACH W/ (2) 16d NAILS AT EACH TRUSS. OVERLAP BRACING AT LEAST ONE TRUSS SPACE (24") PLUS 6" MIN PAST BOTTOM CHORD.

-INSTALL PERMANENT 2x CONTINUOUS LATERAL BRACE (CLB) ACROSS TRUSS WEBS AS INDICATED ON TRUSS SHOP DRAWINGS & BCSI-B3. ("T" BRACING MAY BE USED I.L.O. PEMANENT CLB IF IT EXTENDS MIN. 90% OF THE TRUSS WEB (SEE WEB REINFORCEMENT TABLE WITHIN BCSI-B3).

-AT RAISED HEEL TRUSSES, 2x4 SYP #2 (MIN.) AT TOP AND BOTTOM OF HEEL W/ (2) 12d TOENAILS REQ'D. EXT. SHEATHING MUST BE ATTACHED TO RAISED HEEL TRUSS AND INDICATED BLOCKING. ALTERNATE IF HEEL HEIGHT IS LESS THAN 12" TALL: 2x4 SYP #2 BEHIND THE TRUSS VERT. RUNNING FROM TOP TO BOTTOM ACROSS MAX 4 TRUSSES, THEN BOTTOM TO TOP W/ (2) 12d NAILS AT EACH TRUSS MEMBER (DIAGONAL BRACING) DIAGONAL BRACING REQUIRED AT CLB PER BCSI-B3.

6. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS, WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

FIELD REPAIR NOTES

1. OMITTED REBAR
DRILL A 3/4" Ø HOLE MIN. 6" DEEP AT LOCATION OF OMITTED REBAR, AND INSTALL #5 BAR INTO EPOXY FILLED HOLE. EPOXY TO BE SIMPSON STRONG TIE EPOXY OR EQ. FOLLOW MANU'S INSTRUCTIONS. ASSURE ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM HOLE w/ COMPRESSED AIR PRIOR TO APPLYING EPOXY. ALLOW EPOXY TO CURE TO MANU'S SPECIFICATIONS, THEN FILL CELL IN NORMAL WAY DURING BONDBEAM POUR. USE 30" MIN. LAP SPLICE. HOOKS AT TOP OF BAR TO BE MIN. 12 BAR DIAMETERS(EXCLUDING BEND).
2. STRAPS/CONNECTOR SUBSTITUTION
THE CONTRACTOR MAY SUBSTITUTE ANY PLAN INDICATED STRUCTURAL CONNECTOR MAINTAINING THE SUBSTITUTED CONNECTOR EXCEEDS ALL PUBLISHED VALUES WITH REGARDS TO BOTH UPLIFT AND LATERAL VALUES. ADDITIONALLY, ALL STRUCTURAL CONNECTORS SHALL BE INSTALLED PER THE PRODUCT MANUFACTURERS SPECIFICATIONS.
3. STAIR CONSTRUCTION
2X12'S MAY BE USED ILO TIMBERSTRANDS FOR STAIR STRINGERS. RIM BOARDS MAY BE USED FOR RISERS & TREADS ILO 2x12'S (SEE GEN. STAIR DETAIL SHEET FOR MORE INFO.)
4. METAL STUD SUBSTITUTION
WOOD STUDS MAY BE SUBBED w/ METAL STUDS IN NON-LOAD BEARING WALLS
5. OMITTED J-BOLT
-1/2" ALL THREAD ROD w/ 2" x 2" SQ. WASHER DRILL & EPOXY w/ SIMPSON SET EPOXY WITH MIN. 7" EMBEDMENT.
-1/2"x6" SIMPSON TITEN HD THREADED ANCHORS W/ LBP1/2 WASHER MAY BE USED ILO 1/2" J-BOLT. TAPCON FASTENERS MAY BE USED ILO SIMPSON TITENS
6. MORTAR JOINTS
-FOR BED JOINTS 1/2"-1", GROUT FILL CELLS IN BLOCK AFFECTED (NO VERT. REINF. REQ'D)
-FOR HEAD JOINTS GREATER THAN 3/4", GROUT FILL CELL IN BLOCK AFFECTED ON EA.SIDE OF JOINT. PROVIDE (1) #5 VERT. REINF. IN EA. CELL CONT. PER PLAN.
**EXCEPTION: IF TIE BEAM COURSE AND ADJ. TWO COURSES BELOW, AS WELL AS 1st (2) COURSES FROM FND. HAVE HEAD JOINTS WHICH FALL W/IN THE SPECIFIED TOLERANCES, THE ADDED VERTICAL REINFORCEMENT IS NOT REQ'D TO BE EPOXIED INTO TIE BEAM/FTG
-FOR BLOCKS NOT STAGGERED, GROUT FILL CELL EA. SIDE OF JOINT AND PROVIDE (1) #5 VERT. REINF. IN EA. CELL, CONT. PER PLAN.
7. OVERHANGING CMU WALL
-FOR CMU OVERHANGING FOUNDATION <1/2", NO FIX IS REQ'D POUR TIEBEAM & VERT. CELLS PER PLAN.
-FOR CMU OVERHANGING FOUNDATION FROM +1/2"-1-1/8", GROUT FILL FIRST (3) COURSES IN WALL ALONG AREA AFFECTED & TIE BEAM AND VERTICAL CELLS PER PLAN
-FOR CMU OVERHANGING FOUNDATION OVER 1-1/8", CONTACT EOR FOR REPAIRS.

STRUCTURAL STEEL

1. 1. MATERIAL SPECS:
- WIDE FLANGE SECTIONS: ASTM A992, FY = 50 KSI FU = 65KSI
- TUBE STEEL (HSS): ASTM A500, GRADE B, FY = 42 KSI
- PIPE STEEL: ASTM A53, GRADE B, FY = 35 KSI
- ALL OTHER STRUCTURAL & MISC. STEEL: ASTM A36, FY = 36 KSI
2. STRUCTURAL STEEL CONNECTIONS:
- ALL BOLT CONNECTIONS SHALL BE AT "SNUG-TIGHT" CONDITION AS DEFINED IN AISC 360.
- BOLTS USED IN STEEL CONNECTIONS: A325 U.N.O.
- ANCHOR BOLTS CAST IN CONCRETE: ASTM F1554 GRADE 36
- BOLTS SMALLER THAN 5/8" DIA.: A307
- ALL SHOP AND FIELD WELDS TO USE E70XX ELECTRODE
3. STEEL SHOP DRAWINGS TO BE PROVIDED TO EOR FOR REVIEW AND APPROVAL BEFORE FABRICATION. SHOP DRAWINGS TO INCLUDE ALL SHOP AND ERECTION DETAILS INCLUDING PROFILES, SIZES, SPACING, & LOCATIONS OF STRUCTURAL MEMBERS, CONNECTIONS ATTACHMENTS (INCLUDING WELD SIZE AND TYPE), FASTENERS, LOAD, AND TOLERANCES.
4. STRUCTURAL STEEL SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT FOR AREAS WHICH WILL RECEIVE SPRAY-ON FIRE PROTECTION.
5. A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTION TO ENSURE CONFORMANCE WITH PLANS AND SPECS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

DESIGN PRESSURES

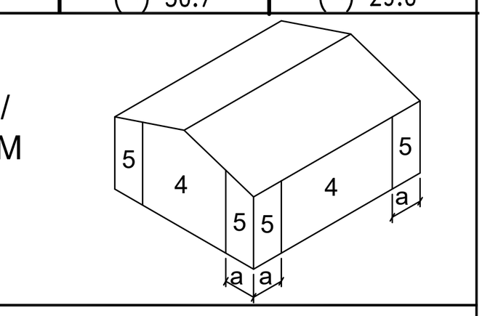
WIND DESIGN LOAD INFORMATION

WIND SPEED (ULTIMATE)	140 mph
WIND SPEED (ALLOWABLE)	112 mph
WIND EXPOSURE	D
RISK CATEGORY	II
BUILDING TYPE	V B
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	+/- 0.18

EFFECTIVE WIND AREA (SQ FT)	WIND PRESSURE & SUCTION (PSF) (+) VALUE DENOTES PRESSURE (-) VALUE DENOTES SUCTION	
AREA	4	5
10	(+) 31.2 (-) 33.7	(+) 31.2 (-) 41.6
20	(+) 29.7 (-) 32.4	(+) 29.7 (-) 38.8
50	(+) 27.9 (-) 32.4	(+) 27.9 (-) 35.2
100	(+) 26.5 (-) 29.1	(+) 26.5 (-) 32.4

GARAGE DOORS	9'-0" x 7'-0"	16'-0" x 7'-0"
	(+) 27.1 (−) 30.7	(+) 26.1 (−) 29.0

WIND PRESSURE/ SUCTION DIAGRAM



STANDARD DESIGN PRESSURE NOTES

1. ASCE 7-10 WALL DESIGN ALLOWABLE COMPONENTS AND CLADDING WIND PRESSURES AND SUCTIONS PER FLORIDA BUILDING CODE 6th EDITION (2017). WIND SPEED IS BASED ON LINEAR INTERPOLATION FOR THIS SITE.
2. MEAN ROOF HEIGHT FOR A TYPICAL SINGLE STORY HOME IS 15FT, 2 STORY HOME IS 30FT
3. MULTIPLY THE ABOVE PRESSURES BY 1.67 TO GET ULTIMATE WIND PRESSURES.
4. "a" = END ZONE IS ONLY W/IN 5'-0" OF ALL EXTERIOR BUILDING CORNERS.
5. INDICATED PRESSURES CAN BE INTERPOLATED FOR OTHER DOOR SIZES, OTHERWISE USE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREAS.
6. DESIGNATED AREAS WHERE THE ULTIMATE WIND SPEED IS 140 MPH OR GREATER IS CONSIDERED TO BE IN THE WIND-BOURNE DEBRIS REGION. GLAZED OPENING PROTECTION IS REQUIRED AND SHALL BE PROVIDED PER SECTION R301.2.1.2 OF THE FLORIDA BUILDING CODE - RESIDENTIAL, CURRENT EDITION.

INDEX OF DRAWINGS

UPDATES

NUMBER	DATE	DESCRIPTION	INTIALS
1	9/14/2019	CREATED CUSTOM PLANS	XXX

THIS RESIDENTIAL STRUCTURE IS DESIGNED TO WITHSTAND THE WIND LOADING IMPOSED IN ACCORDANCE WITH THE LOCATION OF SAID STRUCTURE, PROVISIONS OF CHAPTER 3, AND COMPLIES WITH ALL OTHER PROVISIONS OF THE FLORIDA BUILDING CODE 6TH EDITION (2017) - RESIDENTIAL
DO NOT SCALE THESE DRAWINGS. DIMENSIONS ARE TO BE FOLLOWED AS SHOWN. THE ENGINEER'S SIGNATURE SERVES TO CERTIFY THE STRUCTURAL PORTIONS OF THE PLAN ONLY AND DOES NOT CERTIFY ANY NON-STRUCTURAL ELEMENTS OR DESIGN SPECIFIC COMPONENTS. CONTACT E.O.R. FOR ANY NEEDED CLARIFICATIONS

COLIN F. GARDELLA LICENSE No. 73199 STATE OF FLORIDA PROFESSIONAL ENGINEER

COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

SHEET TITLE

STRUCTURAL NOTES

PROJECT

COHEN RESIDENCE
1204 N. PARK AVE.
WINTER PARK, FL 32789

MONTA CONSULTING & DESIGN
OF WMR AND ASSOCIATES, LLC
CERT. OF AUTHORIZATION NO. 9177
1035 GREENWOOD BLVD., SUITE 173
LAKE MARY, FL 32746
PHONE: (407) 681-1917
www.montaconsulting.com

N · C · B · D · C NATIONAL COUNCIL OF BUILDING DESIGNER CERTIFICATION Certification No. 10-243

AIBD.

"STRENGTH BY DESIGN"

STRUCTURAL ENG:	***
DRAFTING:	
-STRUCT:	***
QC CHECK:	***
JOB No.	ZPROP-0206279
SCALE:	AS INDICATED
DATE:	8.26.19

S1

FOUNDATION PLAN

1/4"=1'-0"

THIS RESIDENTIAL STRUCTURE IS DESIGNED TO WITHSTAND THE WIND LOADING IMPOSED IN ACCORDANCE WITH THE LOCATION OF SAID STRUCTURE, PROVISIONS OF CHAPTER 3, AND COMPLIES WITH ALL OTHER PROVISIONS OF THE FLORIDA BUILDING CODE 6TH EDITION (2017) - RESIDENTIAL
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COLIN F. GARDELLA
LICENSE
No. 73199
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

PROJECT
COHEN RESIDENCE
1204 N. PARK AVE.
WINTER PARK, FL 32789

SHEET TITLE
FOUNDATION PLAN

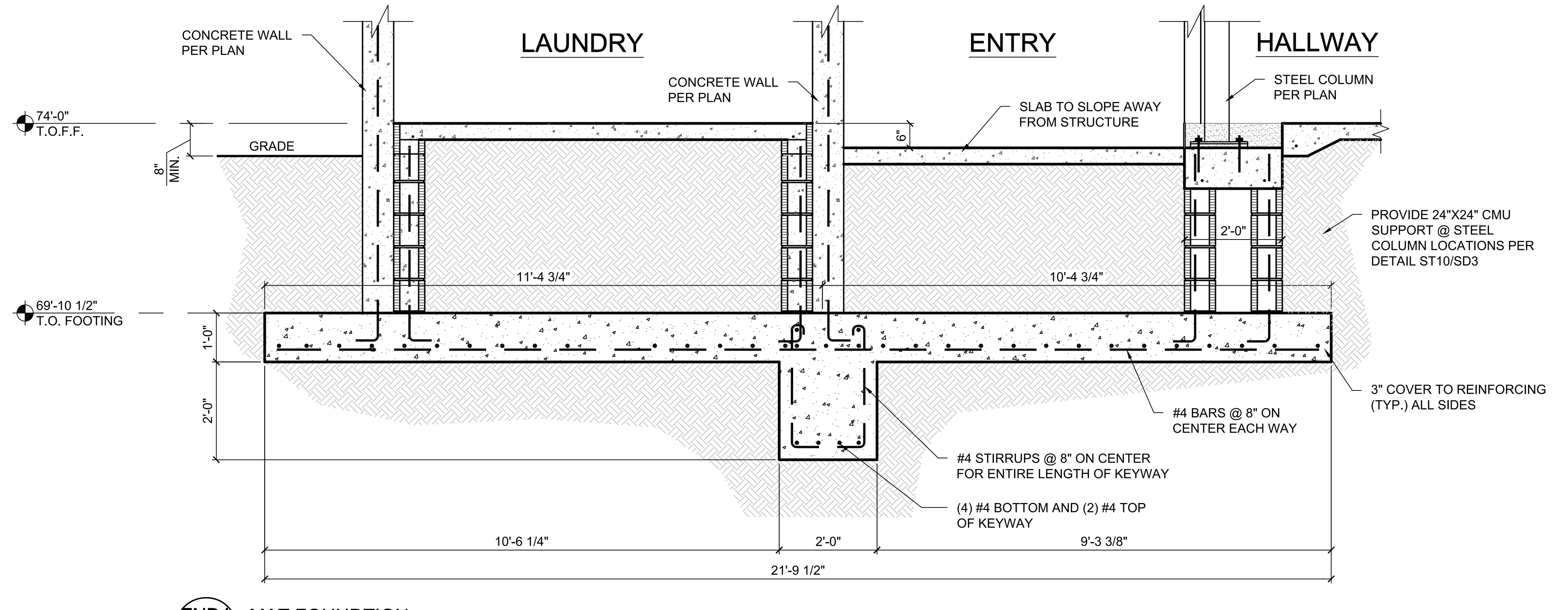
"STRENGTH BY DESIGN"

MONTA CONSULTING & DESIGN
OF WMR AND ASSOCIATES, LLC
CERT. OF AUTHORIZATION NO. 9177
1035 GREENWOOD BLVD., SUITE 173
LAKE MARY, FL 32746
PHONE: (407) 681-1917
www.montaconsulting.com

N · C · B · D · C
NATIONAL COUNCIL OF BUILDING DESIGNER CERTIFICATION
Certification No. 10-243

STRUCTURAL ENG:	***
DRAFTING:	
-STRUCT:	***
QC CHECK:	***
JOB No.	ZPROP-0206279
SCALE:	AS INDICATED
DATE:	8.26.19

S2



FND1 MAT FOUNDTION

1/2"=1'-0"

1. PROVIDE MIN. 6 MIL. APPROVED VAPOR BARRIER. ALL JOINTS TO BE LAPPED MIN. 6" AND SEALED. REFER TO FBC-R CURRENT EDITION SECTION R506.2.3 FOR REQUIRED LOCATIONS
2. PROVIDE 4" THICK 3000 PSI CONC. SLAB W/ 6x6xW1.4xW1.4 WELDED WIRE FABRIC (WWF), PLACED IN MIDDLE TO UPPER 1/3 OF SLAB DEPTH OVER 6 MIL. VAPOR BARRIER ON COMPACTED FILL (FIBER MESH MAY BE USED ILO WWF, WHEN FIBER MESH MEETS OR EXCEEDS SPECIFICATIONS SET FORTH IN CURRENT CODE EDITION).
3. VERT. #4 BAR TO BE HOOKED AT TOP & BOTTOM (FTG. & TIE BEAM ENDS) W/ MIN. 12 BAR DIAMETER LEGS EXCLUDING BEND @ EA. END. LAP SPLICES SHALL BE NO LESS THAN 25".
4. CONSULT W/ MAN. SPECIFICATIONS PRIOR TO POURING OR RECESSING DOOR SILLS. INSWING DOORS WITH COVER (MIN 5FT RADIUS IN ALL DIRECTIONS FOR SIDES OF DOOR) DO NOT NEED A RECESS AT SLAB. IF MIN COVER IS NOT MET IN ANY DIRECTION, A RECESS IS REQ'D & CONTRACTOR TO FOLLOW THE TYP. RECESS DETAIL. INSWING DOORS WITH NO COVER REQUIRE A RECESS
5. EXTERIOR SLABS SHALL SLOPE MIN.1/8" PER FOOT AWAY FROM HOUSE.
6. CONTROL JOINTS (IF SHOWN) ARE NOT REQ'D BY CODE BUT ARE SUGGESTED (ESPECIALLY WHEN USING FIBER REINF. CONC.) CONTROL JOINTS TO BE SAW CUT A DEPTH OF 1/4" OF THE THICKNESS OF THE SLAB. FILL CUT W/APPROVED JOINT MATERIAL OR USE ALTERNATE APPROVED METHOD.
7. MECHANICAL EQUIP. LOCATIONS WILL BE DETERMINED BY COMMUNITY & COUNTY CODES.
8. FOUNDATIONS AND SUPPORTS FOR OUTDOOR MECHANICAL SYSTEMS SHALL BE RAISED AT LEAST 3" ABOVE THE FINISHED GRADE AND SHALL ALSO CONFORM TO THE MANUFACTURER'S INSTALLATION INSTRUCTIONS.
9. NO WOOD STAKES PERMITTED IN FOUNDATION.
10. PENDING SITE CONDITIONS, FOUNDATION MAY HAVE TO BE STEPPED DOWN. SEE SHEET S3 FOR ADDITIONAL INFORMATION. GC TO DETERMINE STEP LOCATIONS IF REQUIRED
11. SEE TYPICAL DETAIL ON LINTEL PLAN FOR REQUIRED STEEL BENDS AND LAP SPLICE. SEE DTL MF09 AND MF10 ON S3 FOR STANDARD FOOTING STEP DOWN AND CORNER INFORMATION.
12. ANY EQUIPMENT AND/OR APPLIANCES HAVING AN IGNITION SOURCE SHALL BE ELEVATED A MIN OF 18". CONTRACTOR TO PROVIDE SUCH PLATFORM w/ EITHER MASONRY OR WOOD CONSTRUCTION
13. ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION: 2000 PSF (SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION REQUIREMENTS). IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET OR EXCEED THE CAPACITY, THE GENERAL CONTRACTOR SHALL CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION OF FOUNDATION DESIGN. SOIL TO BE FREE OF ORGANIC MATERIAL AND COHESIVE SOILS, COMPACTED IN 12" LIFTS TO AT LEAST 95% OF MAX. DRY DENSITY AS DETERMINED BY ASTM - D1557 (MODIFIED PROCTOR).
THE FOUNDATION SIZES INDICATED ON THE FOUNDATION PLAN HAS BEEN DESIGNED FOR A MINIMUM SOIL BEARING CAPACITY OF 2000 PSF.
14. PENDING SITE CONDITIONS, IF STEMWALL IS REQUIRED, G.C. TO DETERMINE REQUIRED COURSES FOR STEMWALL FOUNDATION. SEE STEMWALL CHART ON SD1 FOR REQUIRED REINFORCEMENT AND FOOTING SIZES PENDING DEPTH OF STEMWALL. IF CHART IS NOT PROVIDED, CONTACT EOR TO REVIEW AND PROVIDE
15. IF STEM WALL IS REQUIRED TO BE CONSTRUCTED TALLER THAN THE HEIGHTS PROVIDED ON THE STEM WALL SCHEDULE --- **STOP** --- CONTACT E.O.R. PRIOR TO CONSTRUCTION
16. ALL FOUNDATION PADS WHERE SHOWN ON THE BELOW FOOTING SCHEDULE SHALL BE INSTALLED WHERE THE DEPTH OF THE FOOTING WILL BE MEASURED FROM THE LOWEST SLAB ELEVATION MAINTAINING A MINIMUM DEPTH AS SPECIFIED. THE FOUNDATION PAD MAY BE INSTALLED DEEPER THAN THE INDICATED PENDING SITE CONDITIONS AND/OR LOCATIONS. CONTRACTOR TO FIELD VERIFY ALL MINIMUM FOUNDATION DIMENSIONS AND REINFORCEMENT REQUIREMENTS ARE MET.
17. FIELD TO VERIFY ALL GAS LINE AND PLUMBING LOCATIONS PRIOR TO POURING SLAB.

FOOTING SCHEDULE

1	12"x12"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (2) # 5 E.W.
1A	12"x24"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (2) # 5 PARALLEL TO 24" SIDE @3" O.C. & MIN (3) #5 PARALLEL TO 12" SIDE @6" O.C.
2	24"x24"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (3) # 4 E.W.
2.3	28"x28"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (4) # 4 E.W.
2.5	30"x30"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (4) # 4 E.W.
2.6	32"x32"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (5) # 4 E.W.
3	36"x36"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (5) # 4 E.W.
3.5	42"x42"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (6) # 4 E.W.
3.5A	42"x42"x16" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (6) # 4 E.W.
4	48"x48"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (7) # 4 E.W.
4A	48"x48"x16" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (7) # 4 E.W.
4B	48"x48"x24" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (8) # 4 E.W.
4.3	52"x52"x12" dp w/ #5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (7) # 4 E.W.
4.5	54"x54"x16" dp w/#5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (8) # 4 E.W.
4.5A	54"x54"x24" dp w/#5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (9) # 4 E.W.
5	60"x60"x16" dp w/#5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (9) # 4 E.W.
5.5	66"x66"x16" dp w/#5's EQUALLY SPACED EA. WAY MIN. 2" HORIZ. CLEARANCE BETWEEN BARS & 3" MIN. COVER TO EDGE OF CONC.	MIN (10) # 4 E.W.

SYMBOLS LEGEND

- INDICATES GROUT FILLED CELL WITH (1) VERT. #4 CONT. FTG. TO TIE BEAM (5'-4" O.C. MAX U.N.O.).
- INDICATES GROUT FILLED CELL WITH (1) VERT. #6 CONT. FTG. TO TIE BEAM (5'-4" O.C. MAX U.N.O.).
- INDICATES GROUT FILLED CELL WITH (2) VERT. #4 CONT. FTG. TO TIE BEAM.
- INDICATES GROUT FILLED CELL WITH NO REINFORCEMENT.
- 1/2" J-BOLTS 6" FROM EACH END OF WALL AND 32" O.C. (7" MIN. EMBED) U.N.O.
- INDICATES 45° CORNER BLOCK GROUT FILLED WITH (2) VERT. #4 CONT. FT.G TO TIE BEAM.

THIS RESIDENTIAL STRUCTURE IS DESIGNED TO WITHSTAND THE WIND LOADING IMPOSED IN ACCORDANCE WITH THE LOCATION OF SAID STRUCTURE, PROVISIONS OF CHAPTER 3, AND COMPLIES WITH ALL OTHER PROVISIONS OF THE FLORIDA BUILDING CODE 6TH EDITION (2017) - RESIDENTIAL
DO NOT SCALE THESE DRAWINGS. DIMENSIONS ARE TO BE FOLLOWED AS SHOWN. THE ENGINEER'S SIGNATURE SERVES TO CERTIFY THE STRUCTURAL PORTIONS OF THE PLAN ONLY AND DOES NOT CERTIFY ANY NON-STRUCTURAL ELEMENTS OR DESIGN SPECIFIC COMPONENTS. CONTACT E.O.R. FOR ANY NEEDED CLARIFICATIONS

COLIN F. GARDELLA, LICENSE No. 73199, STATE OF FLORIDA, PROFESSIONAL ENGINEER

COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

SHEET TITLE
FOUNDATION PLAN NOTES AND DETAILS

MONTA CONSULTING & DESIGN
OF WMR AND ASSOCIATES, LLC

PROJECT
COHEN RESIDENCE
1204 N. PARK AVE.
WINTER PARK, FL 32789

MCD STRENGTH BY DESIGN

STRUCTURAL ENG:	***
DRAFTING: -STRUCT:	***
QC CHECK:	***
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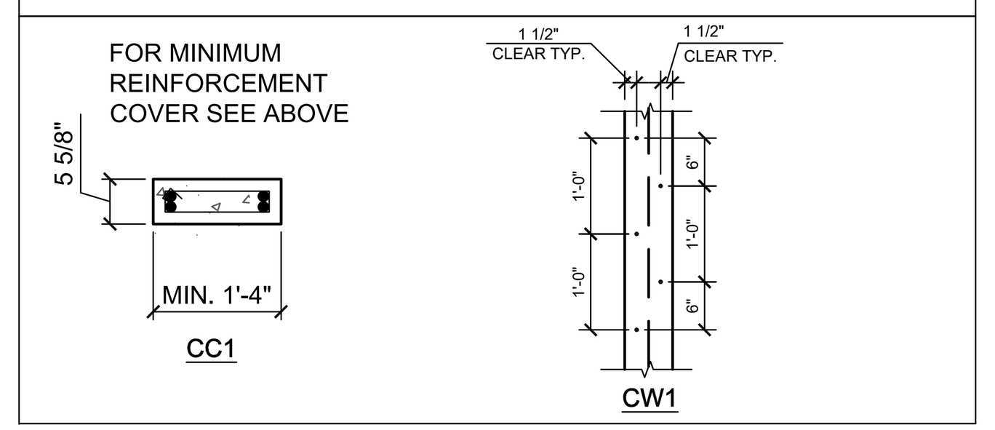
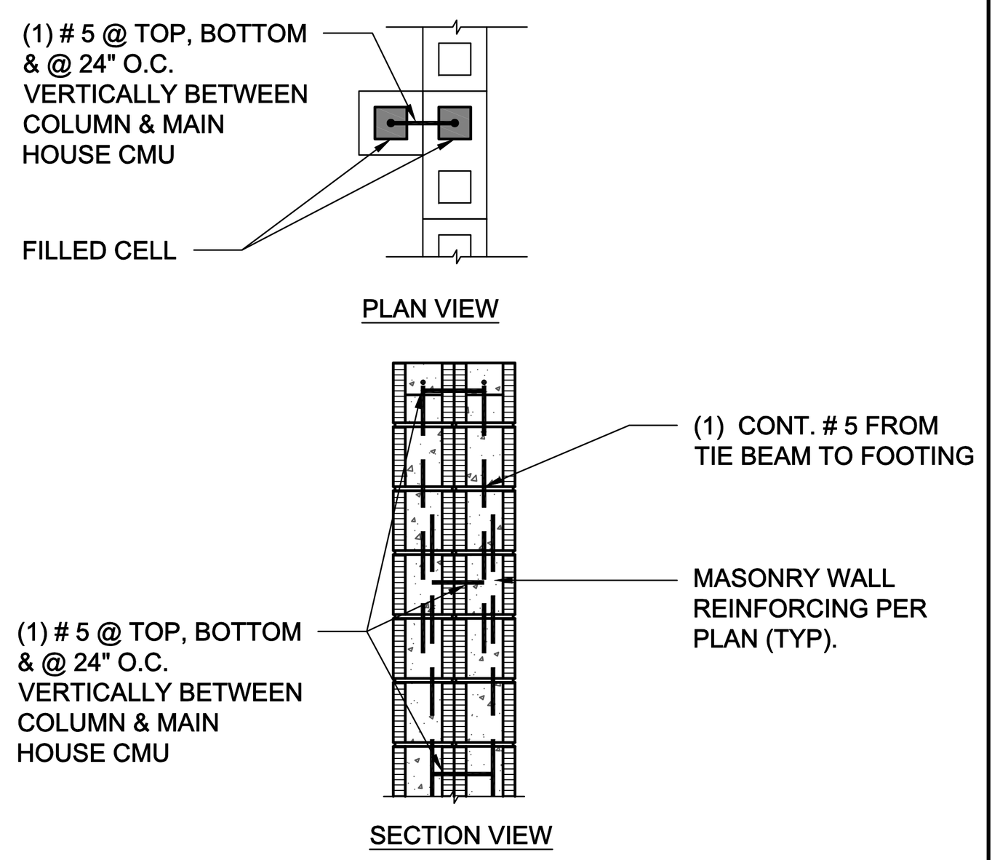
S2.1

STRUCTURAL CONCRETE SCHEDULE

TAG	CC1	CW1
SIZE	5 5/8" x MIN. 16"	7 5/8" x (LENGTH PER PLAN)
VERTS	(4) #4	(2) #4's @ 12" O.C., OFFSET 6" EA. SIDE PER DETAIL
STIRRUPS	#3 @ 8" O.C. (OPEN)	CONT. #4 @ 8" VERT.

1. F'c = 2500 PSI
2. ALL REBAR GRADE 60
3. STIRRUPS GRADE 40
4. ALL VERTICALS TO BE EMBEDDED TO FOOTING BOTTOM REBAR WITH STANDARD 90 DEGREE HOOKS
5. REINFORCEMENT SHALL BE SECURED IN THE PROPER LOCATION IN THE FORMS WITH TIE WIRE OR OTHER BAR SUPPORT SYSTEM SUCH THAT DISPLACEMENT WILL NOT OCCUR DURING THE CONCRETE PLACEMENT OPERATION.
6. MINIMUM COVER FOR REINFORCEMENT IN CONCRETE CAST IN REMOVABLE FORMS SHALL BE 1 1/2" FOR #5 BARS OR SMALLER, AND 2" FOR #6 BARS AND LARGER.
7. NO LOAD SHALL BE APPLIED UNTIL CONCRETE HAS BEEN CURED, AND THE CONCRETE COMPRESSIVE STRENGTH IS EQUAL TO OR GREATER THAN THE SPECIFIED 28TH DAY STRENGTH.
8. CONCRETE SHALL BE DEPOSITED AS NEAR AS PRACTICAL IN ITS FINAL POSITION TO AVOID SEGREGATION DUE TO RE-HANDLING OR FLOWING.
9. AFTER CONCRETING IS STARTED, IT SHALL BE CARRIED ON AS A CONTINUOUS OPERATION, NO JOINTS PERMITTED.





MD10 MASONRY COLUMN TIE IN

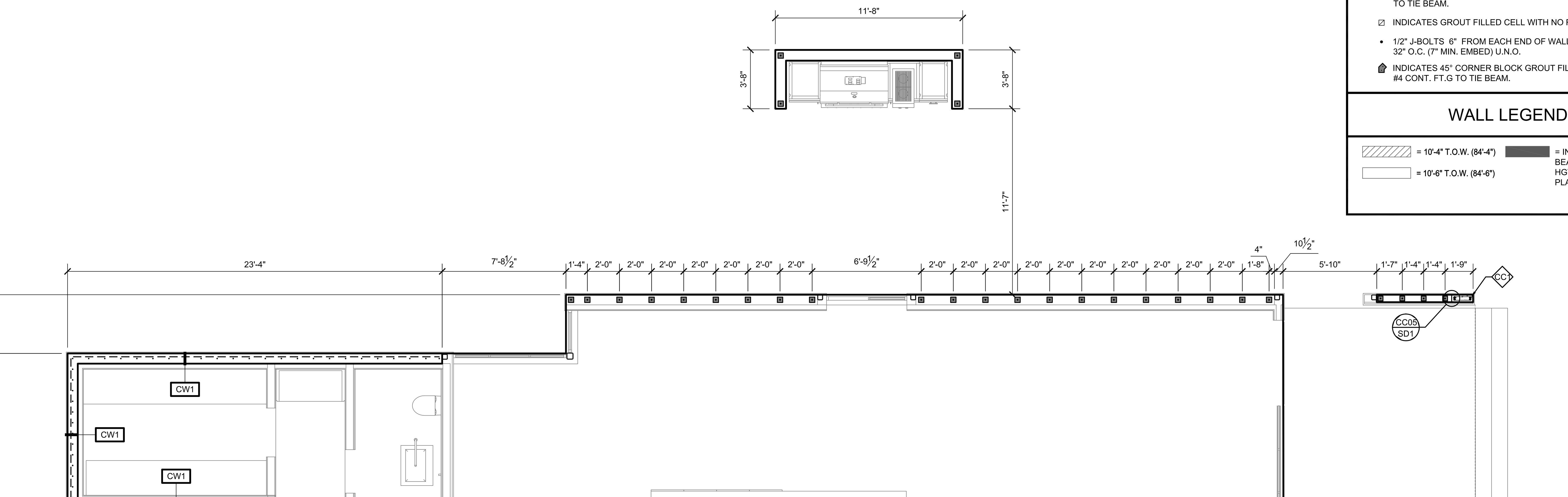
1/2"=1'-0"

SYMBOLS LEGEND

- INDICATES GROUT FILLED CELL WITH (1) VERT. #4 CONT. FTG. TO TIE BEAM (5'-4" O.C. MAX U.N.O.).
- INDICATES GROUT FILLED CELL WITH (1) VERT. #6 CONT. FTG. TO TIE BEAM (5'-4" O.C. MAX U.N.O.).
- INDICATES GROUT FILLED CELL WITH (2) VERT. #4 CONT. FTG. TO TIE BEAM.
- INDICATES GROUT FILLED CELL WITH NO REINFORCEMENT.
- 1/2" J-BOLTS 6" FROM EACH END OF WALL AND 32" O.C. (7" MIN. EMBED) U.N.O.
- INDICATES 45° CORNER BLOCK GROUT FILLED WITH (2) VERT. #4 CONT. FT.G TO TIE BEAM.

WALL LEGEND

- = 10'-4" T.O.W. (84'-4")
- = 10'-6" T.O.W. (84'-6")
- = INTERIOR/EXTERIOR BEARING WALLS VERIFY HGT. W/ ARCHITECTURAL PLAN U.N.O. OR LINTEL




DOWEL PLACEMENT PLAN

1/4"=1'-0"

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COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

SHEET TITLE

DOWEL PLACEMENT PLAN

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AIBD

PROJECT

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MCD "STRENGTH BY DESIGN"

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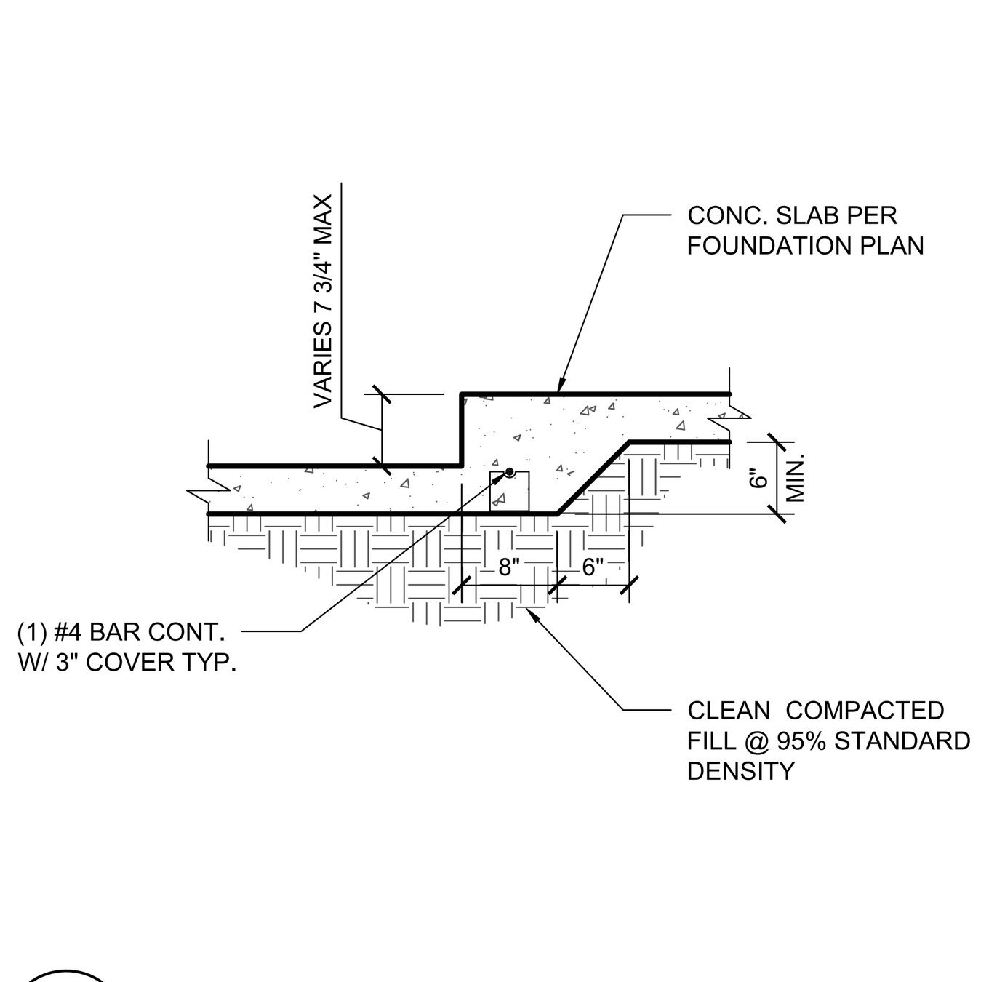
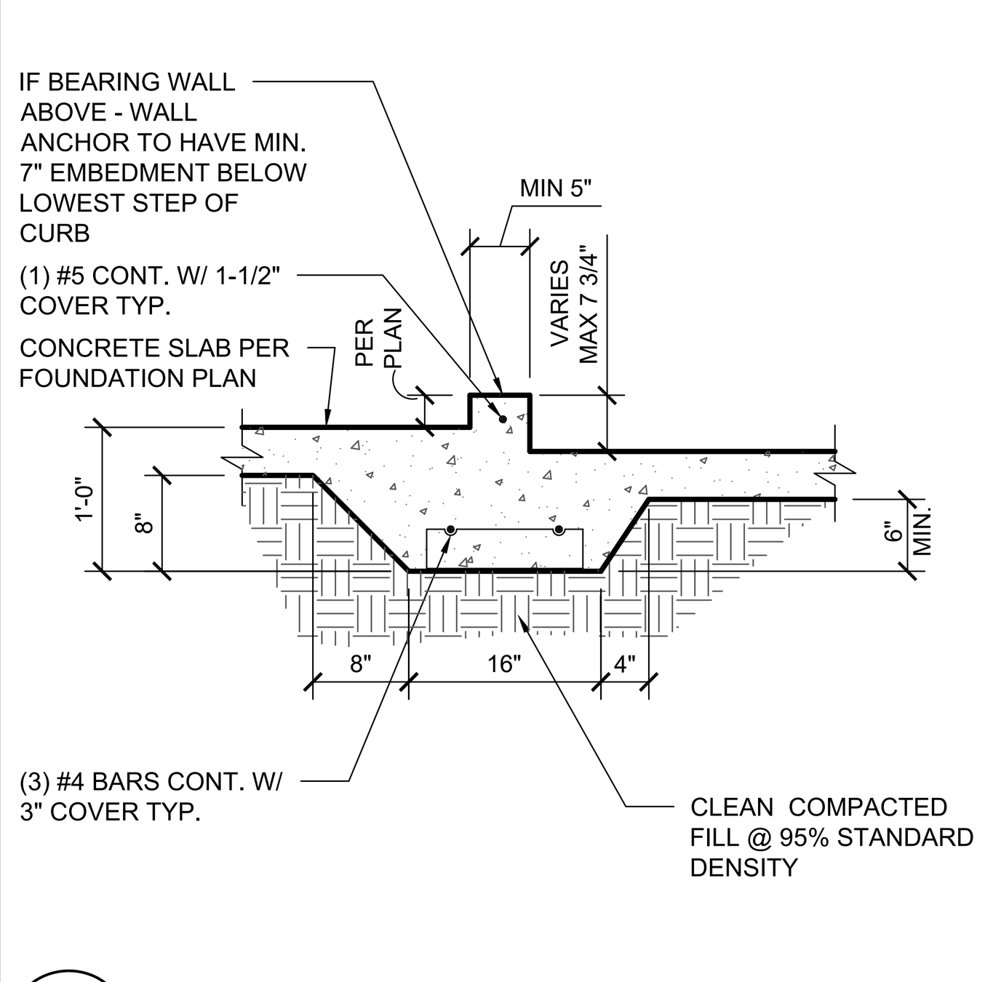
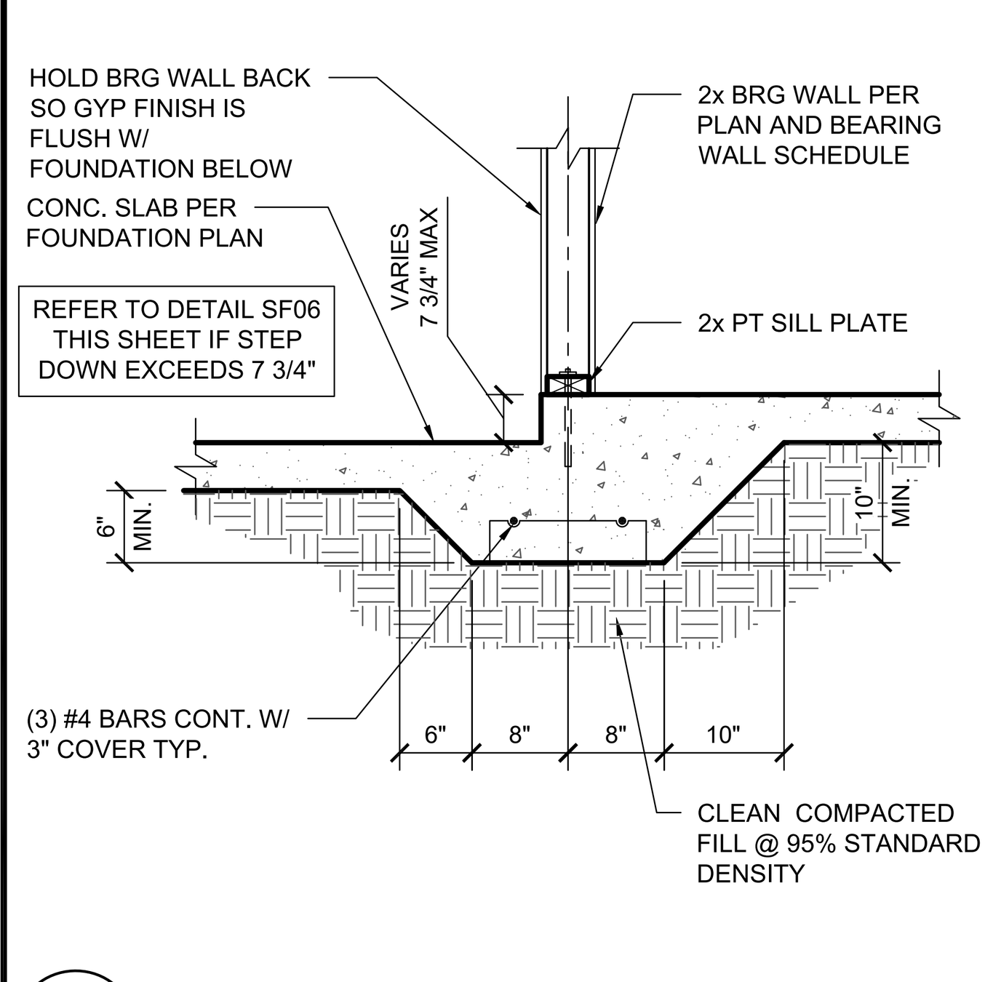
S2.2



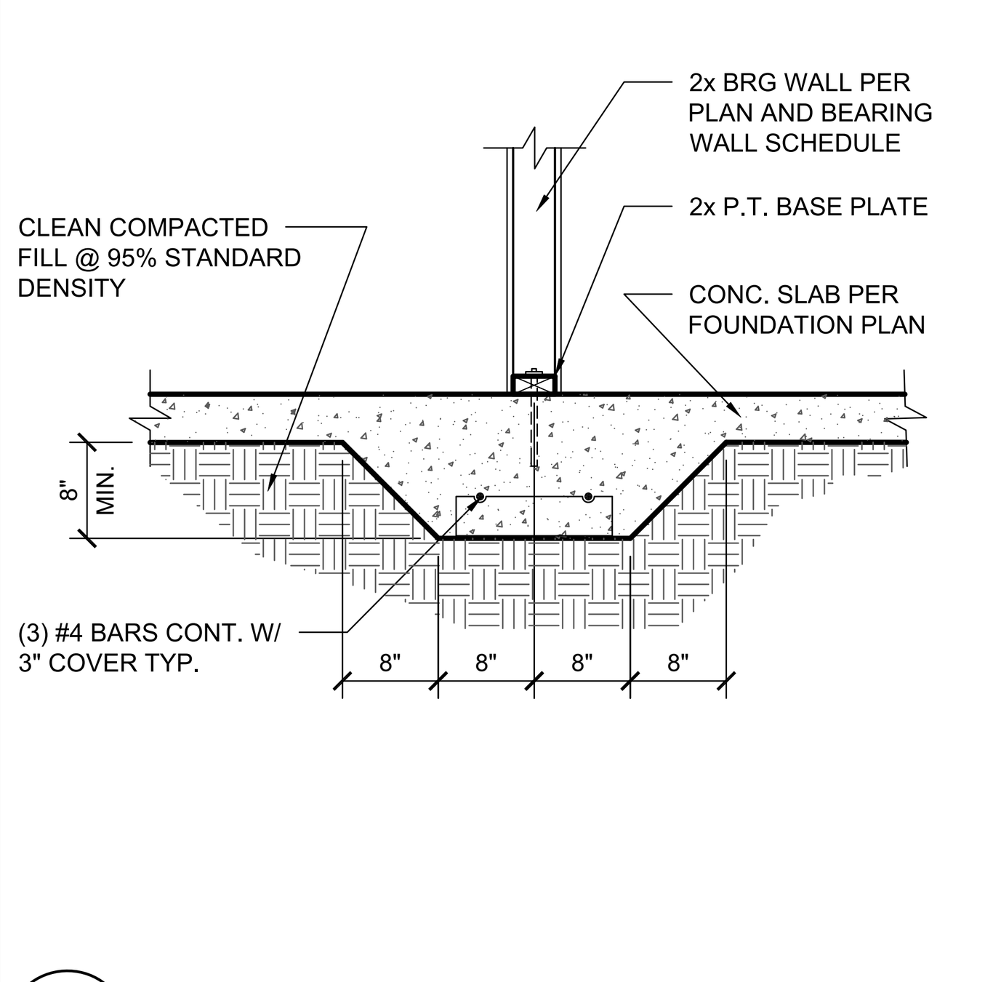
MF03 NON-BEARING SLAB STEP DOWN
3/4"=1'-0"



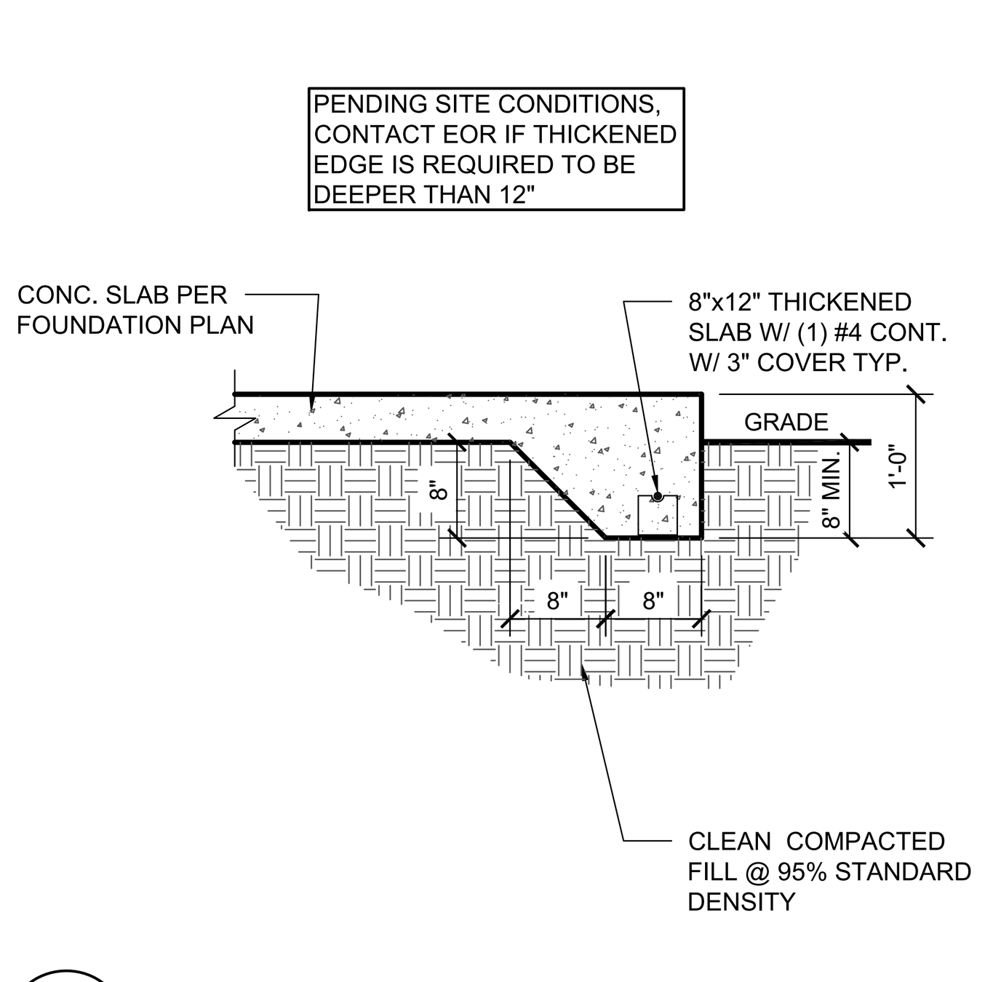
MF04 DOUBLE STEP FOOTING
3/4"=1'-0"



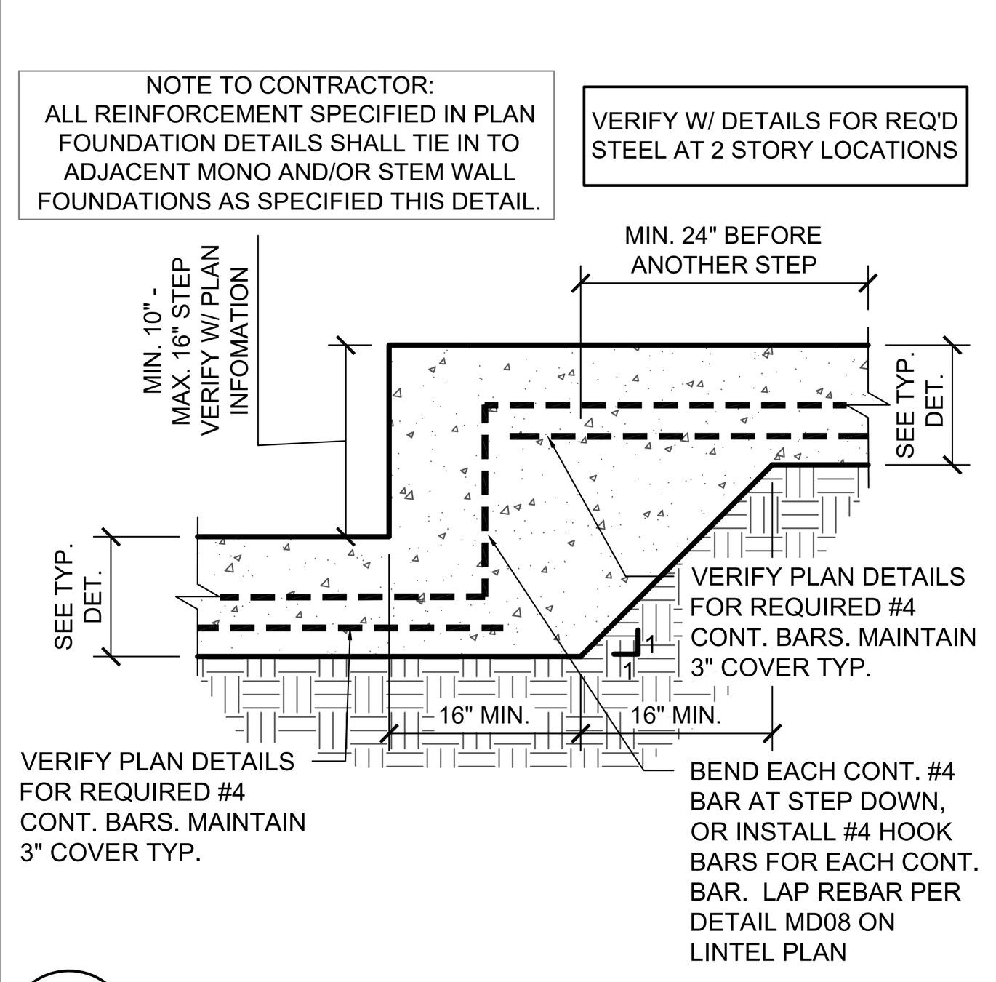
MF05 BEARING SLAB STEP DOWN
3/4"=1'-0"



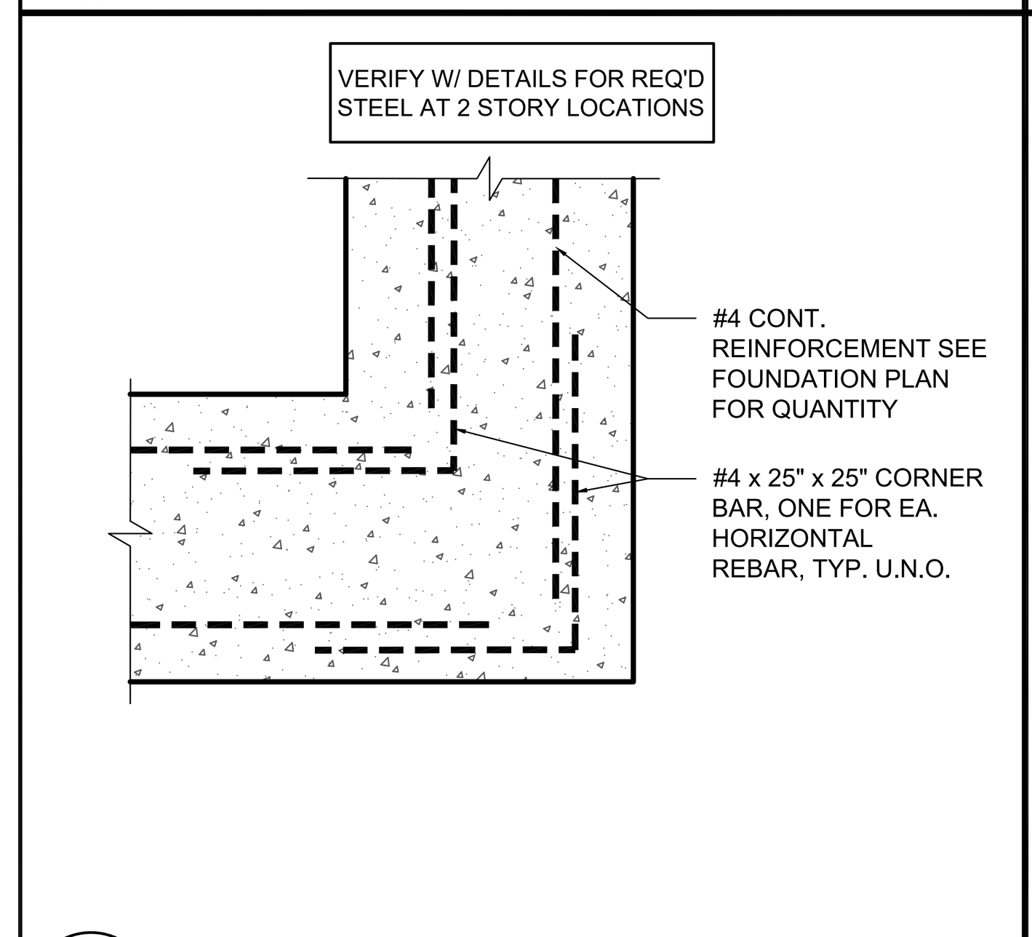
MF06 FOOTING AT INTERIOR BEARING
3/4"=1'-0"



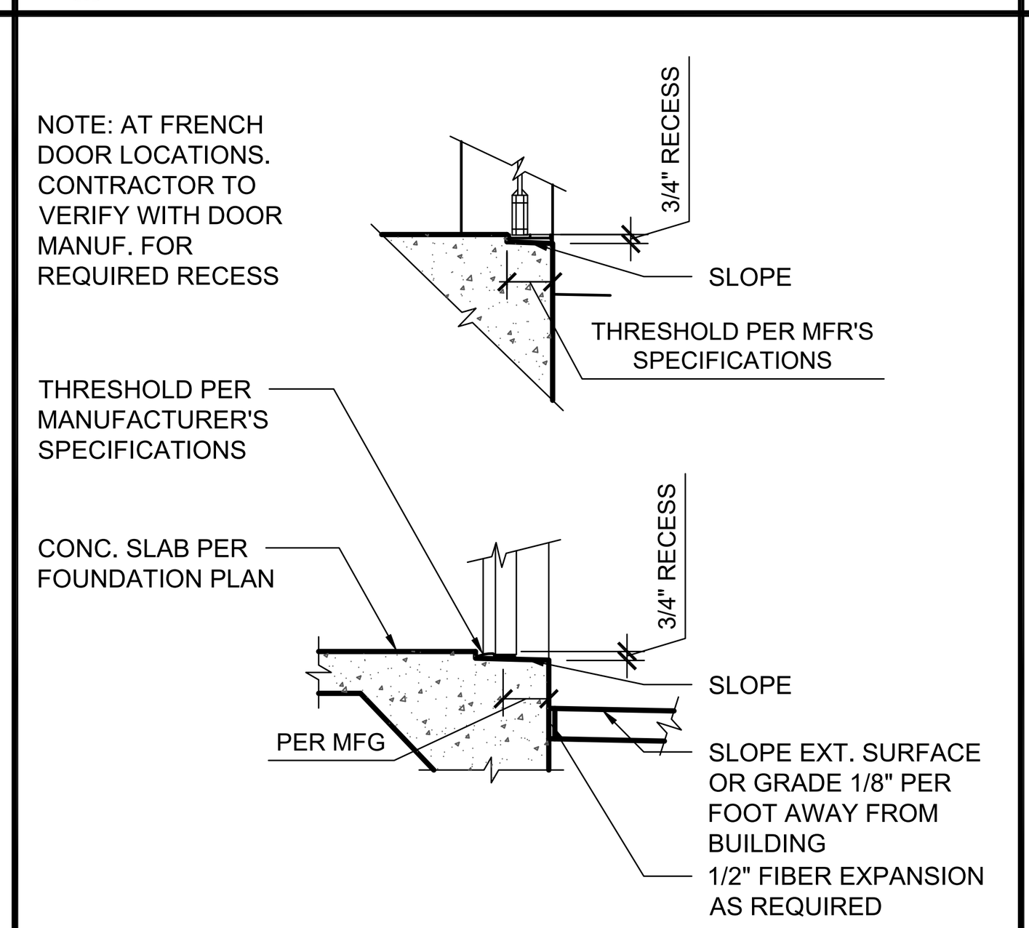
MF08 THICKENED EDGE
3/4"=1'-0"



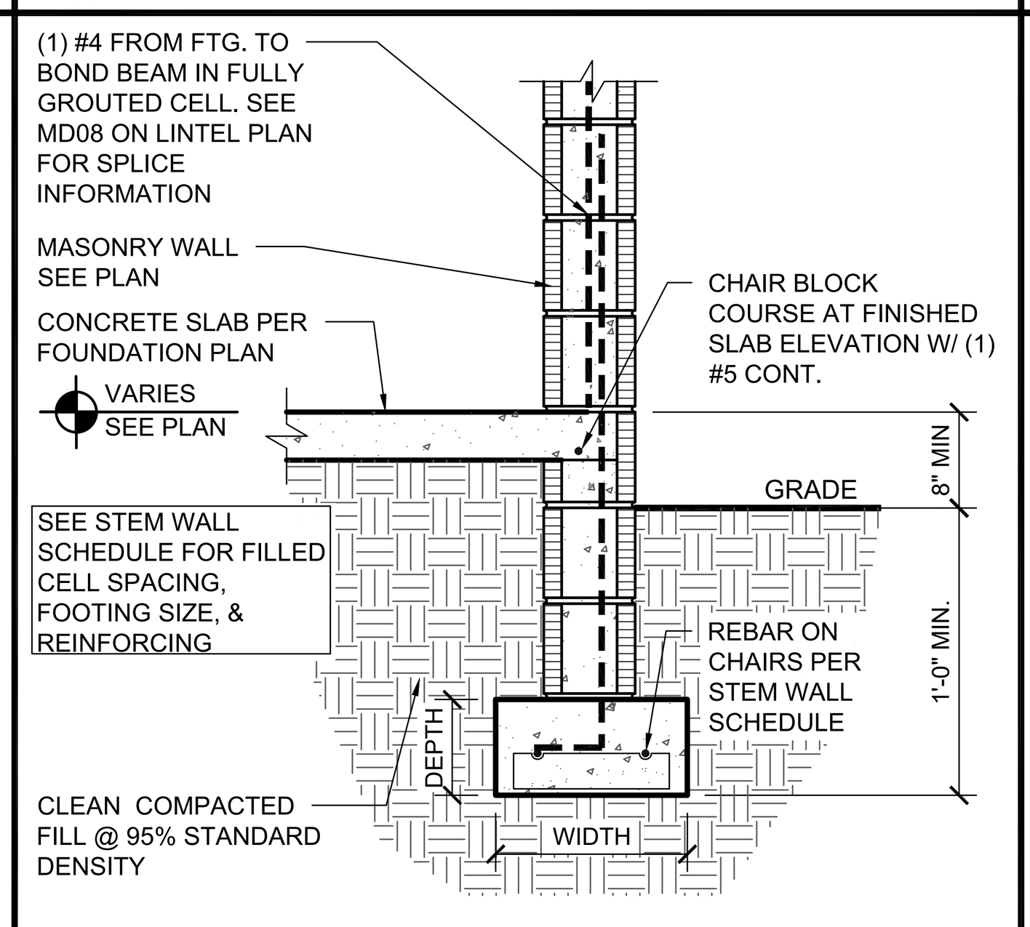
MF09 TYPICAL STEP FOOTING DETAIL



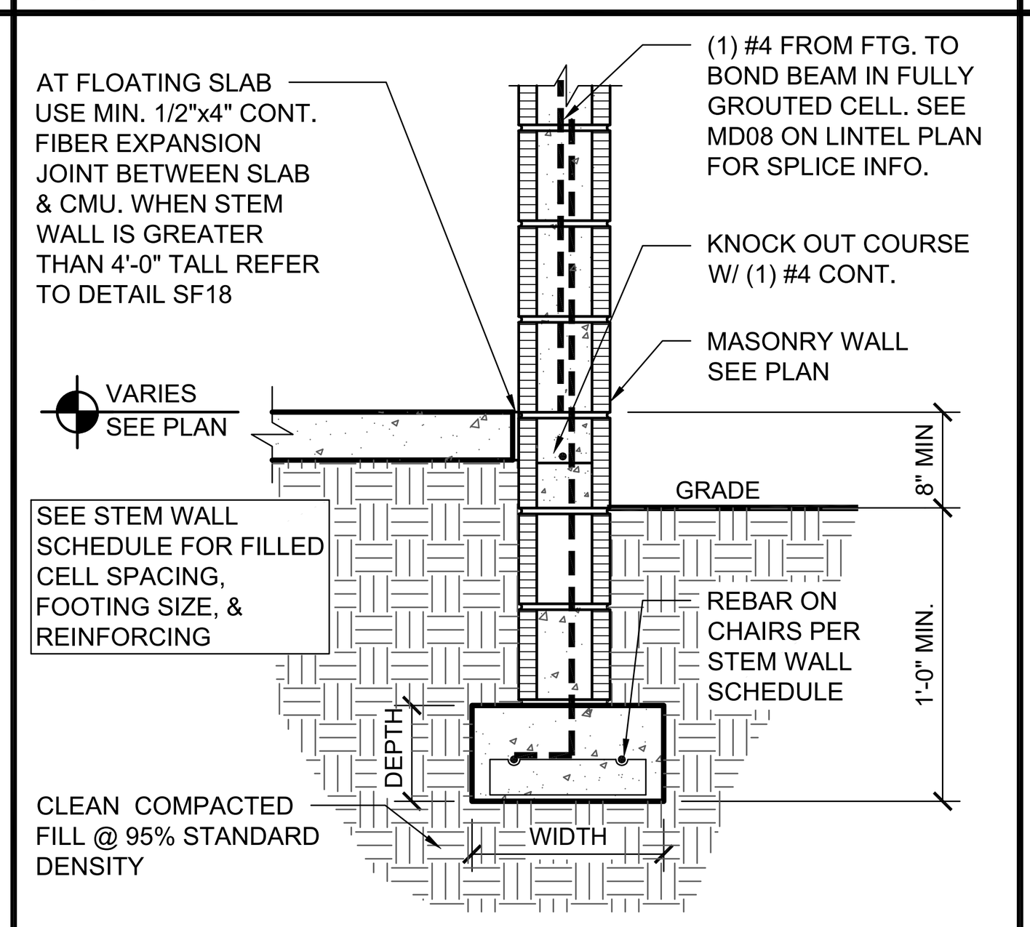
MF10 TYPICAL CORNER BAR DETAIL (TOP VIEW)
3/4"=1'-0"



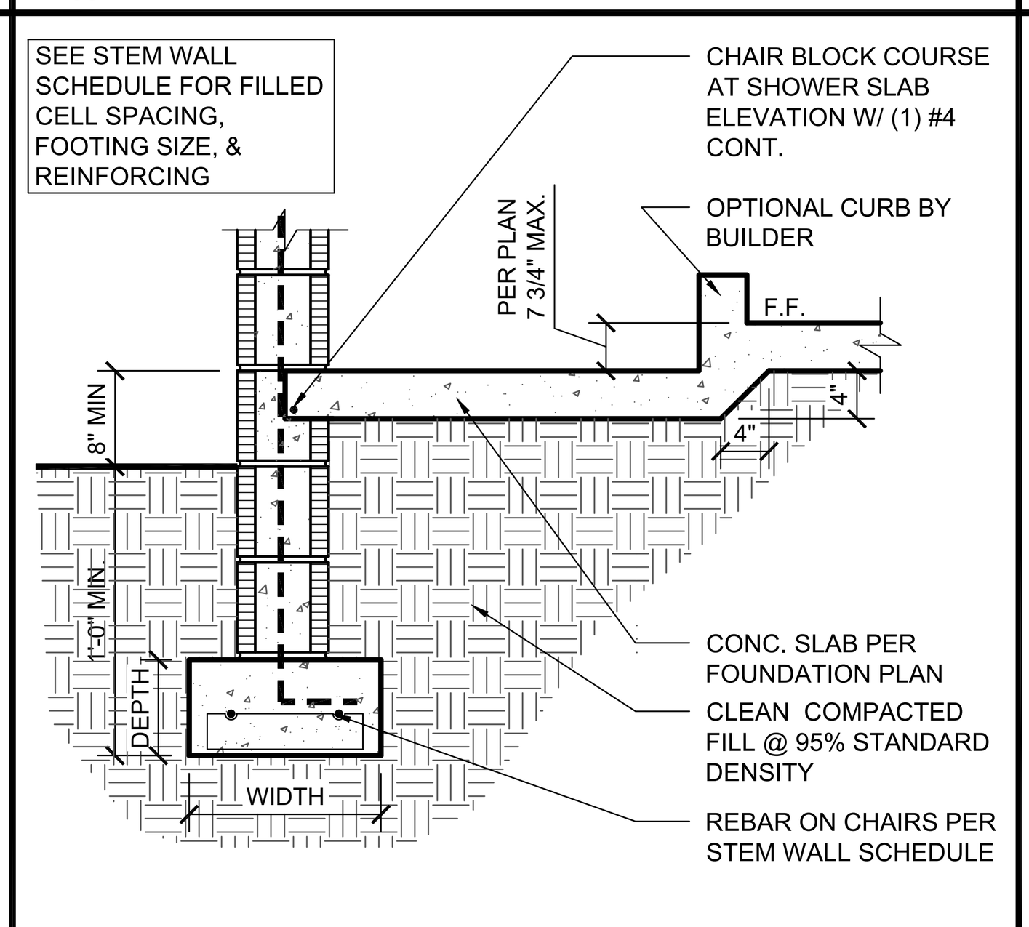
MF11 EXTERIOR DOOR POURED SILL
3/4"=1'-0"



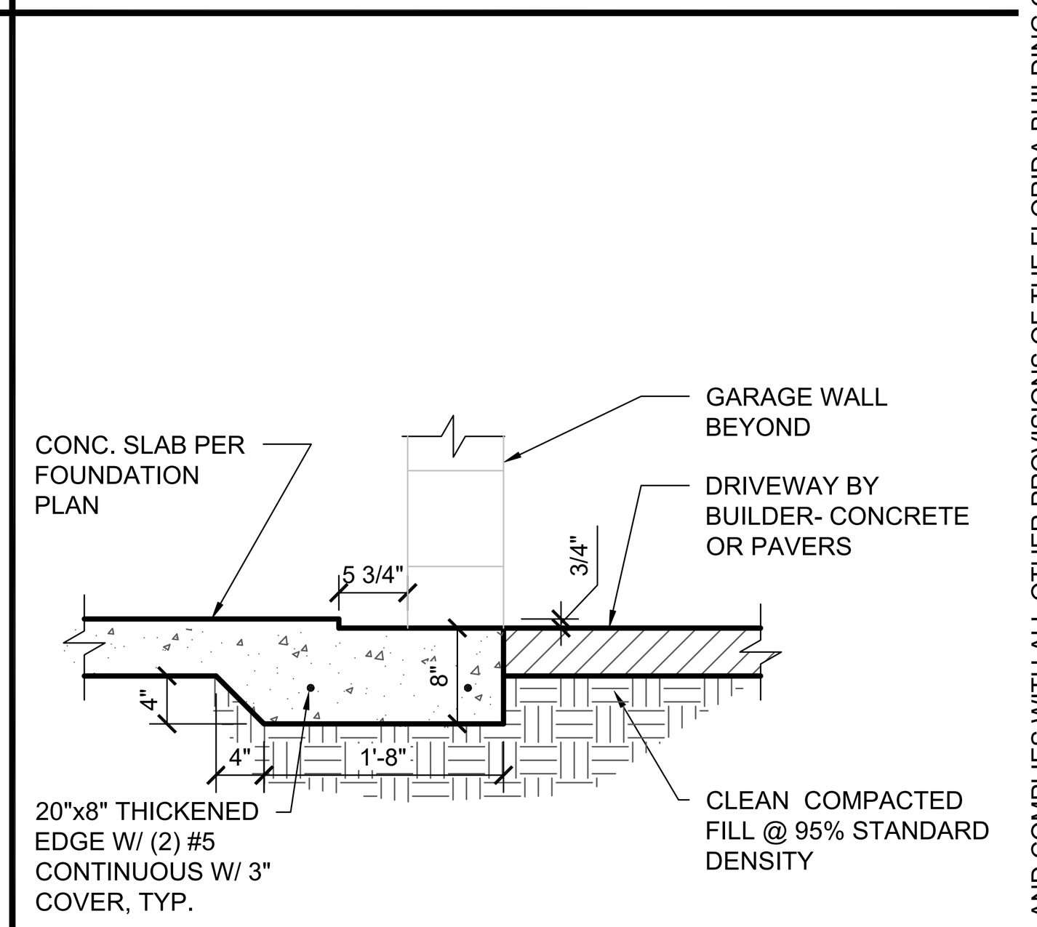
SF01 TYPICAL EXTERIOR FOOTING
3/4"=1'-0"



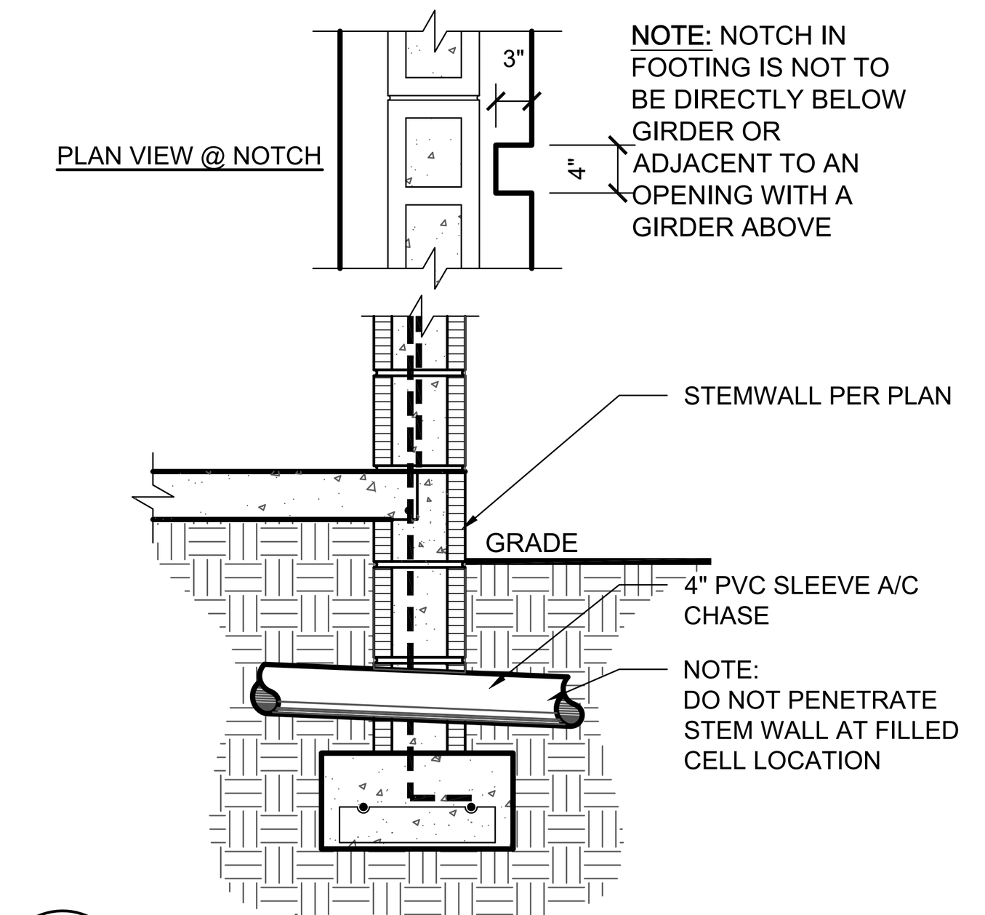
SF02 TYPICAL GARAGE CURB FOOTING
3/4"=1'-0"



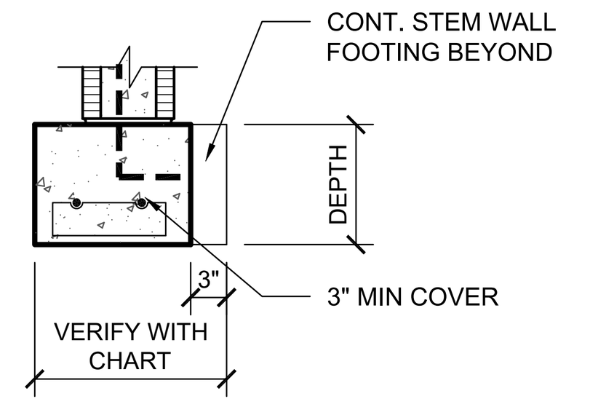
SF03 FOOTING @ SHOWER @ MASONRY
3/4"=1'-0"



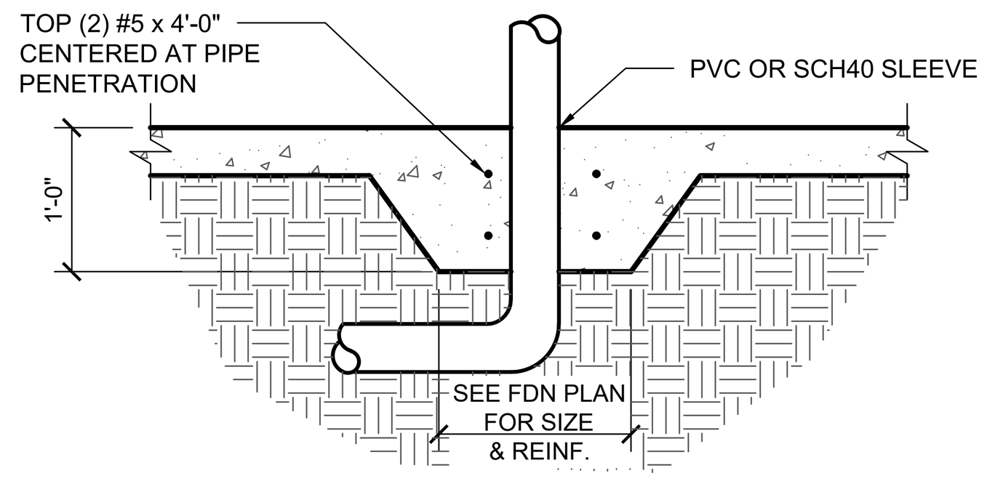
SF04 THICKENED EDGE @ GARAGE DOOR
3/4"=1'-0"



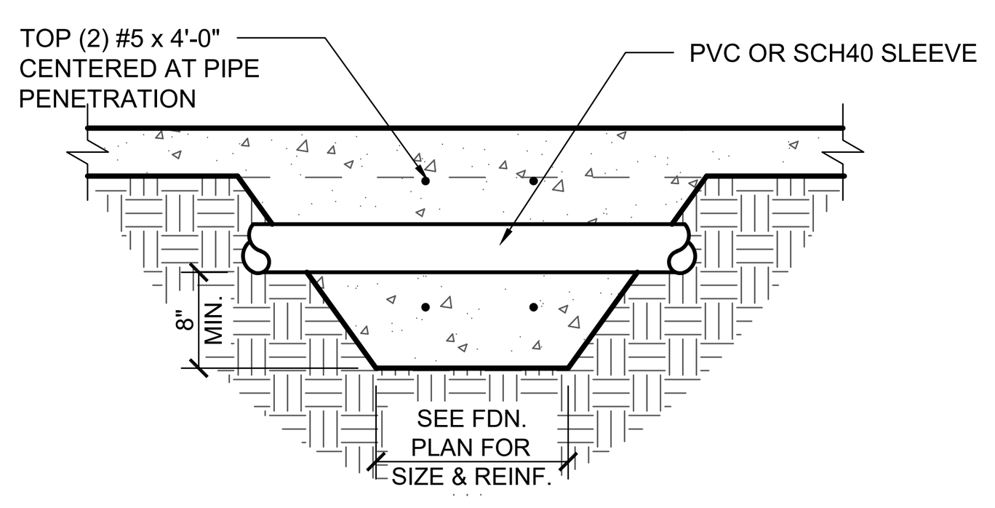
SF10 TYPICAL FOUNDATION PENETRATIONS
3/4"=1'-0"



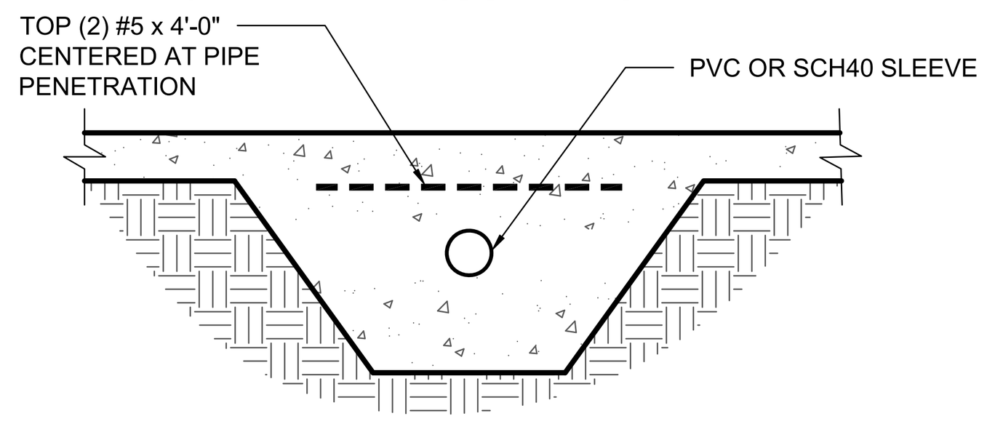
NOTCHED FOOTING @ CONDUIT



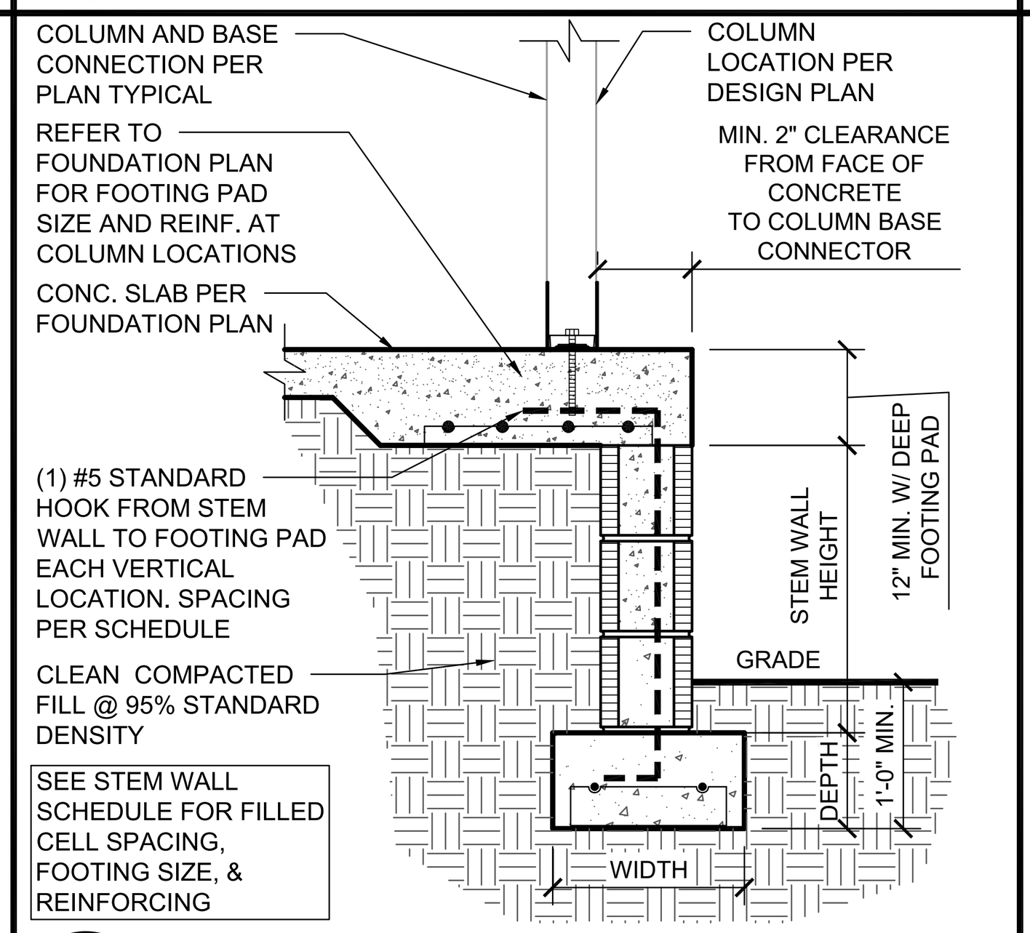
PIPE VERTICAL THRU FDN



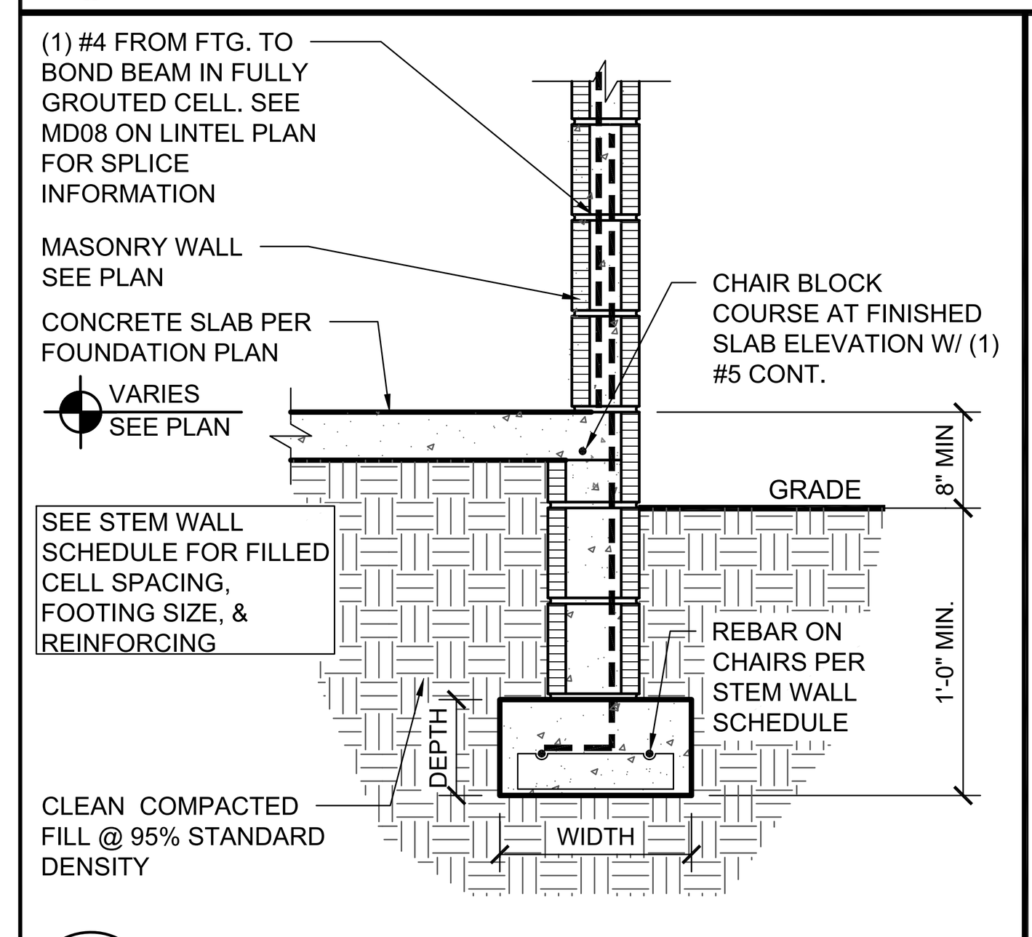
PIPE PERPENDICULAR TO FND



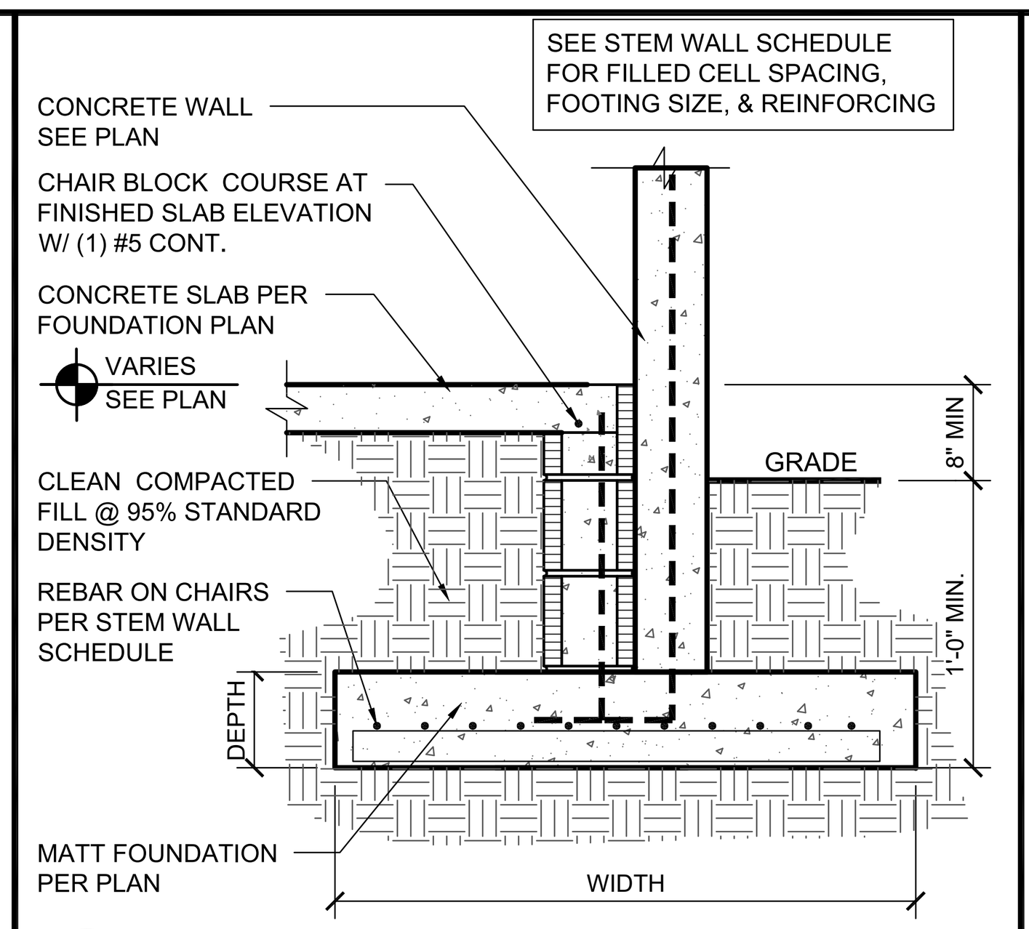
PIPE PARALLEL TO FND



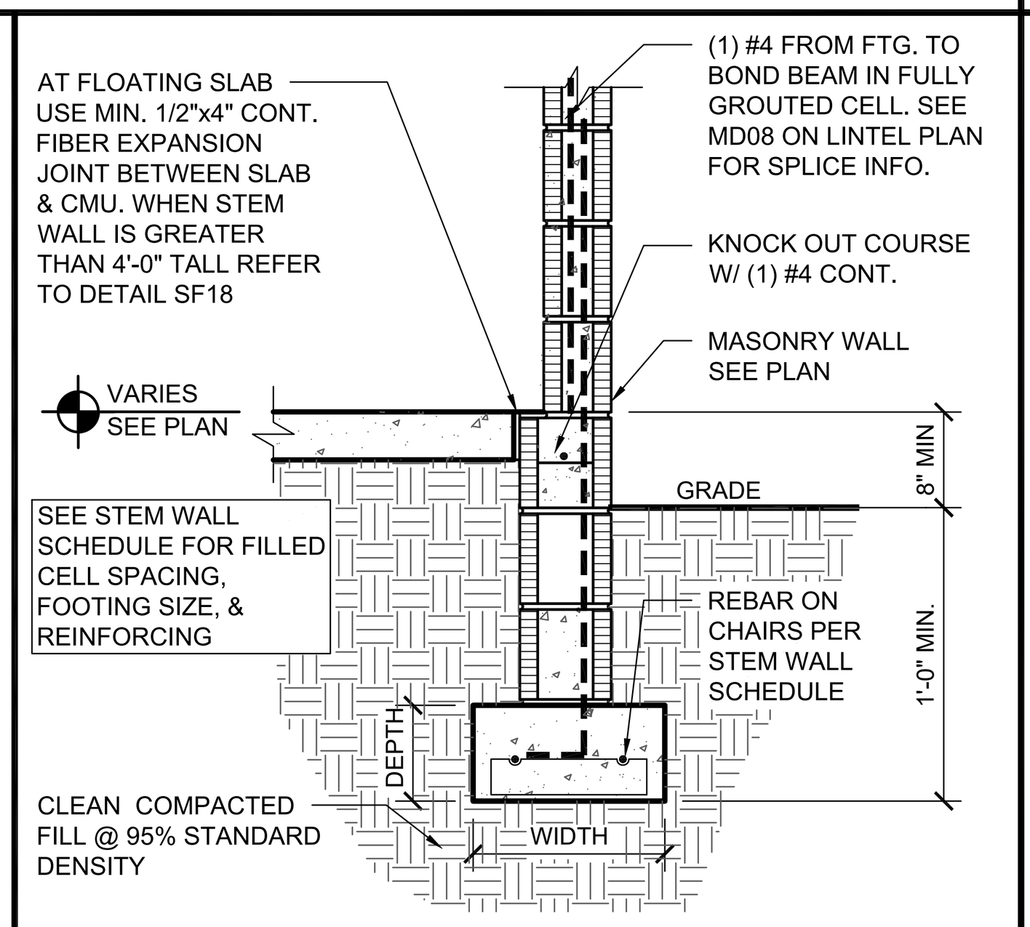
SF07 WOOD COLUMN AT ENTRY
3/4"=1'-0"



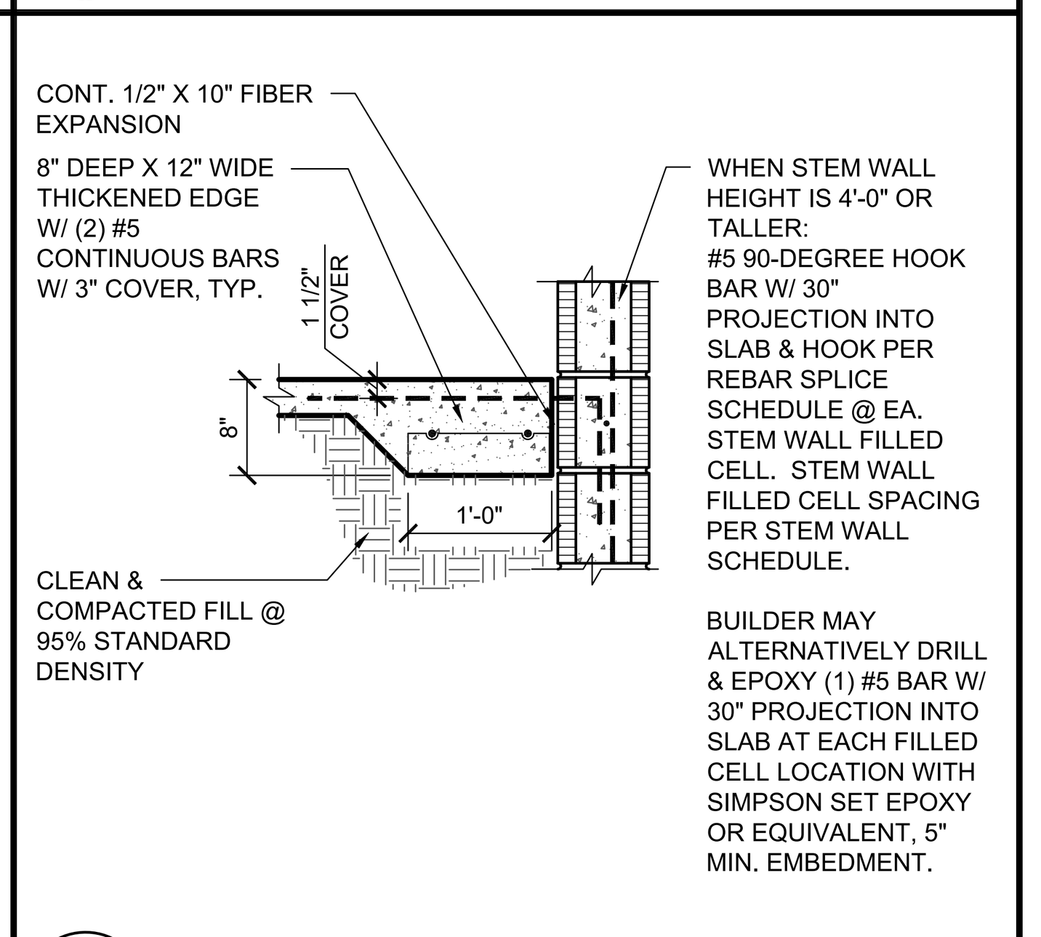
SF01A TYPICAL EXTERIOR FOOTING (6" MASONRY)
3/4"=1'-0"



SF01B TYPICAL EXTERIOR FOOTING (CONCRETE)
3/4"=1'-0"

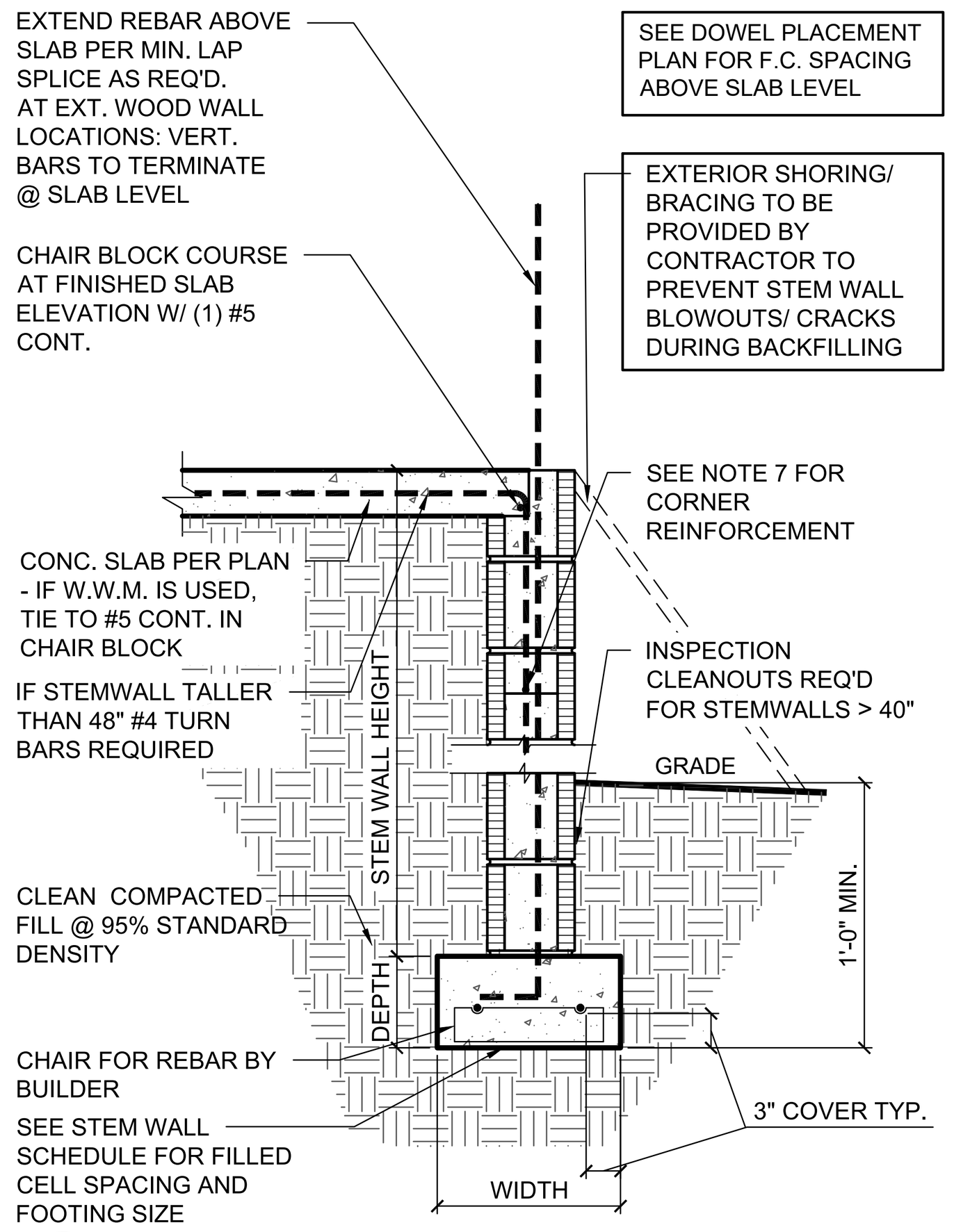


SF02A TYPICAL GARAGE CURB FOOTING (6" MASONRY)
3/4"=1'-0"



SF18 ALTERNATE SLAB TO WALL CONNECTION
3/4"=1'-0"

STEM WALL SCHEDULE				
STEM WALL HEIGHT	FOOTING DIMENSION 1-STORY		NUMBER/ SIZE OF BARS	MAX. FILLED CELL SPACING (O.C.) IN STEM WALL
	DEPTH	WIDTH		
8" - 40"	10"	24"	(4) #5 BARS FOR ONE STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	32	(5) #5 BARS FOR ONE STORY	40"
72" - 80"	10"	36"	(6)#5 BARS FOR ONE	32"



STEM WALL SCHEDULE
3/4"=1'-0"

STEM WALL CONSTRUCTION NOTES

1. VERTICAL REINF. IN SOLID GROUT CELLS AT ALL CORNERS, JAMBS, WALL INTERSECTIONS, BELOW GIRDER TRUSS LOCATIONS, & @ MAX. SPACING STATED IN SCHEDULE U.N.O. ON FOUNDATION/ DOWEL PLAN
2. W.W.M. IN FLOOR SLAB TIED TO THE CONT. #5 BAR IN CHAIR BLOCK IS REQUIRED TO MAKE ADEQUATE CONNECTION BETWEEN SLAB AND WALL. WHEN STEM WALL EXECEEDS 48" IN HEIGHT, W.W.M. ALONE IS NOT SUFFICIENT ENOUGH, AND #4 TURN BARS ARE REQUIRED @ EACH FILLED CELL LOCATION. EACH BAR TO TIE INTO VERTICAL BAR AND EXTEND OUT A MIN. 4'-0" INTO SLAB AND STEM WALL
3. IF STEM IS REQ'D TO BE HIGHER, CONTACT ENGINEER OF RECORD PRIOR TO CONSTRUCTION FOR MORE INFO. NOTE: FILLED CELL SPACING & FOOTING SIZE COULD CHANGE. MCD IS NOT RESPONSIBLE FOR ADDITIONAL CONSTRUCTION COSTS DUE TO REVISIONS FOR TALLER STEM WALLS
4. G.C. TO PROVIDE ADEQUATE BRACING OF STEM WALL WHEN UNEVEN BACK FILLING IS TAKING PLACE
5. IF STEM WALL IS WITH IN 5'-0" OF POOL OR WATER FEATURE FOUNDATIONS, CONTACT EOR FOR FURTHER REVIEW
6. FOUNDATION TO BE CENTERED ON STEMWALL AT ALL TIMES
7. WHEN STEM WALLS ARE GREATER THAN 48" TALL. THE CONTRACTOR IS TO INSTALL MID-HEIGHT KNOCK OUT COURSE WITH (1) #5 HOOK BAR AT ALL CORNERS. HOOK BARS TO BE MIN 4'-0" x 4'-0" IN LENGTH. TIE HOOK BARS INTO VERTICALS @ EA. END, POURED SOLID
8. CONTRACTOR TO CONTACT EOR FOR ADVERSE SITE CONDITIONS NOT COVERED WITHIN THIS DETAIL

CONTRACTOR TO CONTACT EOR. FOR ADDITIONAL REVIEW PRIOR TO THE COMMENCEMENT OF CONSTRUCTION FOR ADVERSE SITE CONDITIONS NOT COVERED WITHIN THE INFORMATION PROVIDED ON THE STEM WALL SCHEDULE

THIS RESIDENTIAL STRUCTURE IS DESIGNED TO WITHSTAND THE WIND LOADING IMPOSED IN ACCORDANCE WITH THE LOCATION OF SAID STRUCTURE, PROVISIONS OF CHAPTER 3, AND COMPLIES WITH ALL OTHER PROVISIONS OF THE FLORIDA BUILDING CODE 6TH EDITION (2017) - RESIDENTIAL
DO NOT SCALE THESE DRAWINGS. DIMENSIONS ARE TO BE FOLLOWED AS SHOWN. THE ENGINEER'S SIGNATURE SERVES TO CERTIFY THE STRUCTURAL PORTIONS OF THE PLAN ONLY AND DOES NOT CERTIFY ANY NON-STRUCTURAL ELEMENTS OR DESIGN SPECIFIC CONDITIONS. CONTACT E.O.R. FOR ANY NEEDED CLARIFICATIONS.

COLIN F. GARDELLA LICENSE No. 73199 STATE OF FLORIDA PROFESSIONAL ENGINEER

COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

SHEET TITLE
FOUNDATION DETAILS

PROJECT
COHEN RESIDENCE
1204 N. PARK AVE.
WINTER PARK, FL 32789

MONTA CONSULTING & DESIGN
OF WMR AND ASSOCIATES, LLC
CERT. OF AUTHORIZATION NO. 9177
1035 GREENWOOD BLVD., SUITE 173
LAKE MARY, FL 32746
PHONE: (407) 681-1917
www.montaconsulting.com

N · C · B · D · C
NATIONAL COUNCIL OF BUILDING DESIGNER CERTIFICATION
Certification No. 10-243

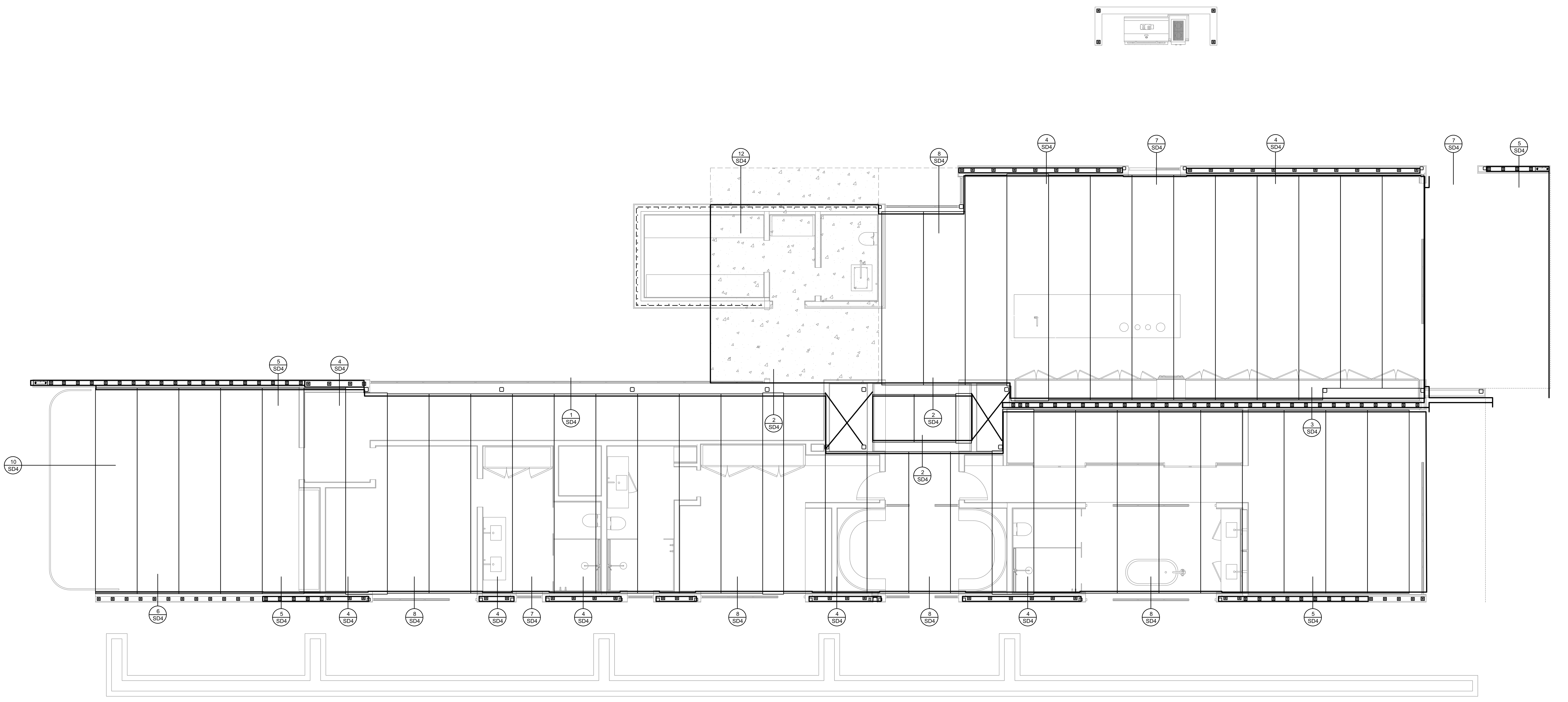
MCD "STRENGTH BY DESIGN"

STRUCTURAL ENG:	***
DRAFTING:	
-STRUCT:	***
QC CHECK:	***
JOB No.	ZPROP-0206279
SCALE:	AS INDICATED
DATE:	8.26.19

S3

WALL LEGEND

Symbol	Meaning
Hatched	= 10'-4" T.O.W. (84'-4")
Open	= 10'-6" T.O.W. (84'-6")
Solid	= INTERIOR/EXTERIOR BEARING WALLS VERIFY HGT. W/ ARCHITECTURAL PLAN U.N.O. OR LINTEL



HOLLOW CORE LAYOUT PLAN

1/4"=1'-0"

NOTE: SEE PAGE S4.2 FOR NOTES AND CONNECTOR LEGEND

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S4

WALL LEGEND

- = 10'-4" T.O.W. (84'-4")
- = 10'-6" T.O.W. (84'-6")
- = INTERIOR/EXTERIOR BEARING WALLS VERIFY HGT. W/ ARCHITECTURAL PLAN U.N.O. OR LINTEL

NOTE: SEE PAGE S4.2 FOR NOTES AND CONNECTOR LEGEND

STEEL LAYOUT PLAN

1/4"=1'-0"

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S4.1

STEEL COLUMN SCHEDULE

A	HSS4x4x3/8 SQUARE TUBE STEEL COLUMN
B	HSS3x3x1/4 SQUARE TUBE STEEL COLUMN
C	HSS5x5x3/8 SQUARE TUBE STEEL COLUMN
D	HSS6x6x3/8 SQUARE TUBE STEEL COLUMN
E	HSS4x2x TUBE STEEL COLUMN
F	(2)2X8 SPF #2 BUILT UP STUD COLUMN
G	(3)2X8 SPF #2 BUILT UP STUD COLUMN
H	(3)2X4 SYP #2 BUILT UP STUD COLUMN

STANDARD NAIL SCHEDULE

STANDARD NAIL	GENERAL DIAMETER/ LENGTH
8d	.131" x $2\frac{1}{2}$"
10d	.148" x 3"
12d	.148" x $3\frac{1}{4}$"
16d	.162" x $3\frac{1}{2}$"
8d RING SHANK	.113" x $2\frac{3}{8}$"

NAILING NOTES

1. CONTRACTOR MAY USE A DIFFERENT NAIL THEN INDICATED AS LONG AS IT MEETS OR EXCEEDS THE DIAMETER AND LENGTH INDICATED.

2. CONNECTORS MAY USE DIFFERENT NAIL LENGTHS. MINIMUM NAIL LENGTH PER CONNECTOR IS REQ. IF LONGER NAIL IS USED IT IS ADEQUATE AS LONG AS IT DOES NOT EXTEND THRU THE MEMBER IT'S BEING ATTACHED TO.

CONNECTOR SCHEDULE

TAG	SIMPSON CONNECTOR	TYPE	VALUES SPF(lb)	VALUES SYP(lb)
1	HETA16 w/(9) 10dx1-1/2" NAILS OR HETA20 w/(9) 10dx1-1/2" NAILS. AT PARALLEL TRUSS CONDITIONS USE LTA2 w/(10) 10dx1-1/12" NAILS @ 24" O.C. U.N.O.	FRAME TO CMU	1450	1450
2	(2)HETA16 OR (2)HETA20 (1)ply TRUSS (10)10d x $1\frac{1}{2}$" NAILS (2)ply TRUSS (12) 16d x $1\frac{1}{2}$" NAILS	FRAME TO CMU	(1)ply 1985 (2)ply 1900	(1)ply 1985 (2)ply 1900
3	H10A w/(18)10d x 1-1/2" NAILS OR (2)ply TRUSS H10A-2 w/(18) 10d x 1-1/2" NAILS	FRAME TO FRAME	H10A 1015 H10A-2 1070	H10A 1340 H10A-2 1245
4	HTS20 w/(24)10d x 1-1/2" NAILS (AT EXT LOCATIONS INCLUDE (3)12d TOENAILS)	FRAME TO FRAME	1245	1450
5	(2)HTS20 w/(24)10d x 1-1/2" NAILS (AT EXT LOCATIONS INCLUDE (3)12d TOENAILS)	FRAME TO FRAME	2490	2900
6	H2.5A w/(10)8d	FRAME TO FRAME	535	600
7	MTS12 w/(14)10d x 1-1/2" NAILS (AT EXT LOCATIONS INCLUDE (3)12d TOENAILS)	FRAME TO FRAME	860	1000
8	MGT w/(22)10d NAILS & (1) 5/8" EPOXY ANCHOR w/ SIMPSON SET-XP EPOXY MIN. 12" EMBEDMENT	FRAME TO CMU	3330	3965
9	HTT4 w/(18)16d x 2-1/2" NAILS & 5/8" EPOXY ANCHOR w/SIMPSON SET-XP EPOXY MIN. 7" EMBEDMENT	FRAME TO CMU	3640	4235
10	HGT-2 w/(16)10d NAILS w/ (2) 3/4" EPOXY ANCHOR w/ SIMPSON SET-XP EPOXY MIN. 12" EMBEDMENT	FRAME TO CMU	10980	8910
11	HTSM16 w/(8)10d NAILS & (4) 1/4"x 2-1/4" TAPCONS	FRAME TO CMU	1010	1175
12	LSTA18 w/(14)10d NAILS	HEADER TO STUDS	1110	1235
13	(2)LSTA18 w/(14)10d NAILS	HEADER TO STUDS	2220	2470
14	LSTA18 w/(14)10d NAILS CENTERED EACH STUD	STUDS TO BEAM/GT	833plf	928plf
15	LSTA24 w/(18)10d NAILS CENTERED EACH STUD	STUDS TO BEAM/GT	1235	1235
16	MSTC28 w/(36)16d SINKERS CENTERED	STUDS TO BEAM/GT	2980	3455
17	MSTC40 w/ (52) 16d SINKERS CENTERED	STUDS TO BEAM/GT	4305	4745
18	LTT20B w/(10) 10d NAILS & $\frac{5}{8}$" EPOXY ANCHOR w/ SIMPSON SET-XP EPOXY MIN. 7" EMBEDMENT	COLUMN TO FTG.	1290	1300
19	HTT4 w/(18) 16d x 2-1/2" NAILS w/ 5/8" EPOXY ANCHOR w/ SIMPSON SET-XP EPOXY MIN. 7" EMBED. AT LVL/PSL COLUMNS USE (18) #10 x 1 1/2" SDS SCREWS INSTEAD OF NAILS	COLUMN TO FTG.	3640	4235
20	(3)HTS20 w/(24)10d x 1-1/2" NAILS (AT EXT LOCATIONS INCLUDE (3)12d TOENAILS)	FRAME TO FRAME	3735	4350
21	MSTCM40 W/ (14) 16d SINKERS AND (10) 1/4"x2-1/4" TITENS	FRAME TO CMU	2420	2800
22	SIMPSON HGUS 5.50/10 W/ (63) 16d NAILS	GIRDER TO COL./ BEAM	9100	4095
23	SIMPSON DTC W/ (4) 8d NAILS TO DOUBLE TOP PLATE & (2) 8d NAILS TO TRUSS. TRUSS TO BEAR ON TOP OF DOUBLE TOP PLATE	FRAME TO FRAME	N/A	N/A

STANDARD CONNECTOR NOTES

1. ALL TRUSS/BEAM TO MASONRY CONNECTIONS TO BE SIMP. HETA16, U.N.O. ON PLAN.

AT FLOOR TRUSSES PARALLEL TO MASONRY WALLS CONNECT w/SIMP. HETA16 @ 24"o.c. OR 32"o.c. VERIFY w/ TRUSS SHOP DRAWINGS FOR VERTICAL SPACING WITHIN FLOOR TRUSS. (IF SITTING ON FRAME WALL USE SIMP. LSTA18(14) @ 24"o.c. OR 32"o.c.)

CONNECT ALL CONTINUOUS RIM BOARDS TO TOP OF MASONRY W/SIMP. LTA2, @ 32"o.c. MAX. AND AT EACH CORNER.

2. ALL TRUSS/BEAM TO FRAME CONNECTIONS TO BE SIMP. H10A FOR SINGLE PLY, OR H10A-2 FOR 2-PLY U.N.O. ON PLAN

3. CONNECT ALL TYPICAL HIP JACKS (CORNER JACKS) TO FRAME USING SIMP. H2.5A CONNECTOR. IF UPLIFT EXCEEDS 535lbs USE (2) H2.5A CONNECTORS.

4. ATTACH GABLE TRUSS TO CANTILEVERED BEAM AT PORCHES USING (2) H2.5A U.N.O. SEE GE01 FOR ADDITIONAL INFORMATION.

5. CONNECT ALL INTERIOR FLOOR TRUSSES/JOISTS TO INTERIOR BRG. WALLS USING (3)12d TOENAILS.

6. ALL TRUSS TO TRUSS CONNECTIONS TO BE SPECIFIED BY TRUSS MANUFACTURER, U.N.O. ON PLAN.

7. MISSED TIE-BEAM STRAPS: AT EACH MISSED HETA16 OR DOUBLE META16 STRAP LOCATION CONTRACTOR TO INSTALL:
(1) SIMPSON HTSM16 FOR TRUSSES WITH 1175 LBS UPLIFT OR LESS.
(2) SIMPSON HTSM16 (INSTALLED ON OPPOSITE FACES OF THE CMU) FOR TRUSSES WITH 1900 LBS UPLIFT OR LESS.
IF MORE THAN (3) TRUSSES IN A ROW HAVE A MISSED META16 CONTACT E.O.R. FOR FURTHER REVIEW.

STANDARD HEADER NOTES

1. VERIFY w/PLAN FOR CORRECT LENGTH OF HEADER REQ'D. SIZE INDICATED ON PLAN IS MINIMUM REQ'D. PER LOCATION. LARGER HEADER MAY BE INSTALLED w/OUT APPROVAL FROM E.O.R.

2. ALL HEADER JACKS/KINGS TO MATCH WALL LUMBER GRADE U.N.O. ON PLAN. ATTACH STUDS TOGETHER FOLLOWING DTL. WF08/SD3.

3. NAIL #2 SYP HEADER PLIES TOGETHER USING:
(2) ROWS OF 10d NAILS @12" O.C. ONE SIDE. MIN. 2" FROM TOP AND BOTTOM FOR 2X6 AND 2X8 HEADERS.
(3) ROWS OF 10d NAILS @12" O.C. ONE SIDE. MIN. 2" FROM TOP AND BOTTOM FOR 2X10 AND 2X12 HEADERS.
NAILING ABOVE IS FOR TOP-LOADED HEADERS ONLY.
CONTACT E.O.R. IF MEMBERS BUCKET INTO HEADER

4. NAIL KING STUDS TO HEADER w/(3) 10d TOENAILS PER SIDE IN ADDITION TO NAILING PER HEADER SCHEDULE.

5. IF MORE THAN (1)KING IS REQ'D. AS PER HEADER SUPPORT TABLE, GC TO INSTALL REQ'D. NAILS FROM 1st KING INTO HEADER THEN ATTACH ADDIT'L KING PER DTL. WF08/SD3. OTHERWISE, CONTACT E.O.R. FOR REQ'D. WOOD SCREWS.

6. IF HEADER IS NOT SPECIFIED CONTACT E.O.R.

7. SEE STRUCTURAL COVER SHEET FOR LVL SPECIFICATIONS.

8. SEE DTL. WF09/SD2 FOR 1ST FLOOR HEADER CONSTRUCTION AND CONNECTIONS AT 2 STORY CONDITIONS. SEE 2ND FLOOR NOTES FOR MORE INFO.

9. FOR BEARING OR EXTERIOR WALLS THAT REQUIRE 2x6 OR 2x8 STUDS. FRAMER TO INSTALL ADDITIONAL HEADER PLY AND FLITCH PLATE AS REQUIRED TO FILL OUT THE WALL. USE (2) ROWS OF 16d NAILS AT 12" O.C. EACH SIDE TO NAIL HEADER TOGETHER.

10. AFP 1.6E SHORTSPAN HEADERS SHALL HAVE THE FOLLOWING MINIMUM SPECIFICATIONS: E = 1600000 psi, FB = 1500 psi, FV = 175 psi, FC-PERP = 650 psi.

11. WHERE SYP #2 BUILT-UP HEADERS ARE SPECIFIED ON THE PLANS, AFP 1.6E SHORTSPAN HEADERS OF THE SAME SIZE MAY BE SUBSTITUTED.

12. AFP 1.6 SHORTSPAN HEADERS MAY NOT BE USED WHERE BUILT-UP LVL HEADERS ARE SPECIFIED ON THE PLANS.

BEAM SCHEDULE

TAG	BEAM SIZE	CONNECTIONS
B2x8	(2)2x8 #2 SYP WITH 1/2" FLITCH PLATE	TO POST: SIMP. HTS20 OR MSTA18 TO BEAM: SIMP. HUC48 w/(14)16d & (6)-10d NAILS TO MASONRY: SIMP. HU48 OR HUC48 w/(6)10d NAILS & (14)$\frac{1}{4}$"x$2\frac{3}{4}$" TAPCONS ON MASONRY: (1) SIMP. MSTAM24
B2x10	(2)2x10 #2 SYP WITH 1/2" FLITCH PLATE	TO POST: SIMP. HTS20 OR LSTA18 TO BEAM: SIMP. HUC410 w/(18)16d & (10) 10d NAILS TO MASONRY: SIMP. HU410 OR HUC410 w/(10)10d NAILS & (18)$\frac{1}{4}$"x$2\frac{3}{4}$" TAPCONS ON MASONRY: (1) SIMP. MSTAM24
B2x12	(2)2x12 #2 SYP WITH 1/2" FLITCH PLATE	TO POST: SIMP. HTS20 OR LSTA18 TO BEAM: SIMP. HUC410 w/(18)16d & (10) 10d NAILS TO MASONRY: SIMP. HU410 OR HUC410 w/(10)10d NAILS & (18)$\frac{1}{4}$"x$2\frac{3}{4}$" TAPCONS ON MASONRY: (2) SIMP. MSTAM24
B3.5x11.25	(2)1.75"x11.25" LVL	TO POST: SIMP. HTS20 OR LSTA18 TO BEAM: SIMP. HUC410 w/(18)16d & (10) 10d NAILS TO MASONRY: SIMP. HU410 OR HUC410 w/(10)10d NAILS & (18)$\frac{1}{4}$"x$2\frac{3}{4}$" TAPCONS ON MASONRY: (2) SIMP. MSTAM24
B3.5x11.875	(2)1.75"x11.875" LVL	TO POST: SIMP. HTS20 OR LSTA18 TO BEAM: SIMP. HUC410 w/(18)16d & (10) 10d NAILS TO MASONRY: SIMP. HU410 OR HUC410 w/(10)10d NAILS & (18)$\frac{1}{4}$"x$2\frac{3}{4}$" TAPCONS ON MASONRY: (2) SIMP. MSTAM24
B3.5x16	(2) 1.75"x16" LVL	TO POST: SIMP. HTS20 OR LSTA18 TO BEAM: SIMP. HUC410 w/(18)16d & (10) 10d NAILS TO MASONRY: SIMP. HU410 OR HUC410 w/(10)10d NAILS & (18)$\frac{1}{4}$"x$2\frac{3}{4}$" TAPCONS ON MASONRY: (2) SIMP. MSTAM24
B5.25x16	(3) 1.75"x16" LVL	TO POST: (2) SIMP. HTS20 OR LSTA18 ON MASONRY: (2) SIMP. MSTAM24 * U.N.O.FASTEN LVL PLIES TOGETHER USING (3) ROWS OF 1/4" X 3 1/2" SDS SCREWS @ 24" O.C. EACH SIDE
B3.5x20	(2) 1.75"x20" LVL	TO POST: (2) SIMP. HTS20 OR LSTA18 ON MASONRY: (2) SIMP. MSTAM24 * U.N.O.FASTEN LVL PLIES TOGETHER USING (4) ROWS OF 1/4" X 3 1/2" SDS SCREWS @ 24" O.C. EACH SIDE

STANDARD BEAM NOTES

1. NAIL ALL (2) 2x SYP BEAMS TOGETHER USING (3) ROWS OF 12d NAILS @ 12" O.C. (TYP. ONE SIDE) MIN. 2" FROM TOP AND BOTTOM OF BEAM

2. NAIL ALL 2-PLY LVL BEAMS (MAX. 12" DEEP) TOGETHER USING (3) ROWS OF 12d NAILS @ 6" O.C. (TYP. ONE SIDE) MIN 2" FROM TOP AND BOTTOM OF BEAM

3. VERIFY w/PLAN CORRECT LENGTH OF BEAM REQ'D. (MIN. 4" BRG. EA. END)

4. SEE PLAN FOR TOP OR BOTTOM OF BEAM INDICATIONS.

5. BEAMS ARE NOT TO BE DRILLED OR NOTCHED IN ANY WAY WITHOUT WRITTEN
APPROVAL FROM THE E.O.R.

6. SEE STRUCTURAL COVER SHEET FOR LVL SPECIFICATIONS

HEADER SCHEDULE

TAG	HEADER	FASTENERS
H2x6	(2)2x6 #2 SYP w/$\frac{1}{2}$" FLITCH PLATE	ATTACH KING STUD TO HEADER w/(6) 16d NAILS
H2x8	(2)2x8 #2 SYP w/$\frac{1}{2}$" FLITCH PLATE	ATTACH KING STUD TO HEADER w/(8) 16d NAILS
H2x10	(2)2x10 #2 SYP w/$\frac{1}{2}$" FLITCH PLATE	ATTACH KING STUD TO HEADER w/(10) 16d NAILS
H2x12	(2)2x12 #2 SYP w/$\frac{1}{2}$" FLITCH PLATE	ATTACH KING STUD TO HEADER w/(10) 16d NAILS
H3.5x5.5	(2)1.75"x5.5" LVL	ATTACH KING STUD TO HEADER w/(6) 16d NAILS
H3.5x7.25	(2)1.75"x7.25" LVL	ATTACH KING STUD TO HEADER w/(8) 16d NAILS
H3.5x9.25	(2)1.75"x9.25" LVL	ATTACH KING STUD TO HEADER w/(10) 16d NAILS
H5.5x11.25	(3)1.75"x11.25" LVL	ATTACH KING STUD TO HEADER w/(12) 16d NAILS
H3x12	(3) 2x12 #2 SYP w/ $\frac{1}{2}$" FLITCH PLATE BETWEEN PLIES	ATTACH KING STUD TO HEADER w/(10) 16d NAILS

HEADER SUPPORT SCHEDULE

OPENING SIZE	SEE PLAN FOR WALL WIDTH JACKS @ EA. END	KINGS @ EA. END
1'-0" TO 3'-11"	1	1
4'-0" TO 7'-11"	2	2
8'-0" TO 9'-11"	2	3
10'-0" TO 12'-0"	3	4

ROOF NOTES:

1. THE ROOF PLAN DEPICTED IS NOT INTENDED TO SERVE AS A TRUSS DESIGN.
2. TOP PLATE HEIGHTS VARY. SEE BUILDING SECTIONS, WALL SECTIONS AND ELEVATIONS FOR BEARING HEIGHTS.
3. TRUSS SPACING SHALL BE 24" O.C. MAX. UNLESS OTHERWISE NOTED. CONVENTIONAL FRAMING SHALL BE 16" O.C. MAX. OR AS OTHERWISE NOTED.
4. FRAME WALLS UP TO UNDERSIDE OF ROOF TRUSSES AT ALL NON-BEARING WALLS AND AT VOLUME AREA UNLESS NOTED OTHERWISE. STUDS CAN BE 24" O.C. W/ SINGLE TOP AND P.T. BASE PLATE LEAVE MAX. 1-1/2" GAP FROM TOP PLATE TO TRUSS BOTTOM CHORD
5. ALIGN TRUSSES AND HAND FRAMING SO ALL GYPSUM WALL BOARD WILL BE CONTINUOUS FROM FLOOR TO CEILING.
6. TRUSS MANUFACTURER TO INSURE DESIGN CONSIDERATION TO THE FOLLOWING ADDITIONAL LOADS:
   A) ALL CEILING HUNG SOFFITS AND SOFFITS WITH CABINETS AS SHOWN ON PLANS.
   B) ATTIC LOCATED HVAC UNITS AS SHOWN ON PLANS.
7. REFER TO MANUFACTURER SPECIFICATIONS FOR INSTALLATION REQUIREMENTS OF ALL HARDWARE BEFORE INSTALLATION.
8. IT IS REQUIRED TO BRACE AND BLOCK PER BSCI 1 CURRENT EDITION. IN ADDITION, PROVIDE BRACING AND BLOCKING SHOWN ON PLANS.
9. ALL TRUSS AND TRUSS CONNECTORS ARE TO BE PROVIDED BY TRUSS SUPPLIER. THIS INCLUDES BUT IS NOT LIMITED TO PIGGY BACK TRUSSES AND VALLEY TRUSSES
10. USE 5/8" GYPSUM BOARD ON CEILING IN GARAGE WHEN LIVING SPACE IS ABOVE THE GARAGE.
11. IF ROOF TRUSS LAYOUT SHOWS TRUSS ID'S, THIS LAYOUT HAS BEEN PROVIDED BY A TRUSS COMPANY TO USE FOR THE DESIGN OF THIS PROJECT. OTHERWISE AN ENGINEERED LAYOUT HAS BEEN DETERMINED BUT PRIOR TO CONSTRUCTION OR TRUSS FABRICATION, FINAL TRUSS LAYOUT AND TRUSS SHOP DRAWS ARE TO BE SUBMITTED TO ENGINEER OF RECORD (E.O.R.) FOR REVIEW AND APPROVAL. AT THIS TIME THE E.O.R. RESERVES THE RIGHT TO REVISE THE PLAN AS REQUIRED PER THE REVIEW OF THE FINAL TRUSS LAYOUT AND TRUSS SHOP DRAWS, ADDITIONAL FEE'S MAY APPLY. STARTING CONSTRUCTION OR TRUSS FABRICATION PRIOR TO THIS REVIEW IS NOT ADVISED, AND THE E.O.R. IS NOT RESPONSIBLE FOR ADDITIONAL COSTS DUE TO REVISIONS OF THE PLAN. IF CONVENTIONAL FRAMING IS SHOWN, NO TRUSS APPROVAL IS REQUIRED, UNLESS LAYOUT IS REVISED W/OUT WRITTEN APPROVAL FROM MCD.

BUILDER NOTE:

IF THE TRUSS LAYOUT SHOWN DOES NOT MATCH THE TRUSS MANUFACTURERS LAYOUT

----STOP----

AND CALL THE ENGINEER OF RECORD PRIOR TO PLACEMENT OF ANY TRUSSES

ROOF & EXT. WALL NAILING SHEATHING NOTES

ROOFS:
ROOF SHEATHING TO BE 5/8" OR 19/32" PLYWOOD (WITH TILE) FASTENING WITHIN 4'-0" OF EAVES TO BE 3" O.C. AT ALL SUPPORTED EDGES AND 8" O.C. IN FIELD. FOR ALL OTHER AREAS NAIL @ 4" O.C. AT ALL SUPPORTED EDGES AND 8" O.C. IN FIELD. SHEATHING TO BE PLACED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS (TRUSSES). PROVIDE PLY CLIPS AT ALL UNSUPPORTED ROOF SHEATHING JOINTS EXCEPT AT PITCH CHANGE LOCATIONS. PLY CLIPS TO BE SPACED NO MORE THAN 24" O.C.

GABLE ENDS:
ALL SHEATHING TO BE MIN. 15/32" O.S.B. AND SHALL BE NAILED (1) ROW @ 3" O.C. AT TOP PLATE AND (1) ROW AT BOTTOM PLATE OF WALL, 6" O.C. ALL OTHER EDGES AND 12" IN FIELD. BLOCKING SHALL BE PLACED AT ALL SHEATHING JOINTS.

EXTERIOR WALLS:
ALL EXTERIOR WALLS TO BE 15/32" OSB SEE WF10/SD3 FOR MORE INFO.

NAILS:
NAILS USED IN ROOF SHEATHING APPLICATIONS SHALL BE 8d RING SHANK NAILS. ALL OTHER SHEATHING APPLICATIONS SHALL BE 8d,8d RING OR SPIRAL SHANK, HAND OR GUN DRIVEN NAILS. GUN DRIVEN NAILS SHALL HAVE HEAD SIZE EQUIVALENT TO HAND DRIVEN NAILS. FASTENERS FOR PRESERVATIVE-TREATED WOOD SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE OR COPPER. THE COATING WEIGHTS SHALL BE IN ACCORDANCE WITH ASTM A 153.

APA SHEATHING NOTE:
24/16 SPAN RATED SHEATHING REQUIRES BLOCKING AT ALL UNSUPPORTED PANEL EDGES AT HIP LOCATIONS. BLOCKING TO BE 2X4 ATTACHED AT EACH END WITH (3) 10d TOENAILS.

FLOOR FRAMING NOTES

GENERAL NOTES

1. FLOOR JOIST/TRUSS HANGERS SUPPLIED & ENGINEERED BY FLOOR JOIST/TRUSS CO.
2. REFER TO TRUSS MANU. DRAWINGS FOR TRUSS DESIGNATIONS.
3. GENERAL FLOORING FINISHES ARE ACCEPTABLE. IF LIGHTWEIGHT CONCRETE OR SELF LEVELING CONCRETE IS REQ. CONTACT E.O.R. ALONG WITH TRUSS CO. TO VERIFY FLOOR TRUSS DESIGN.
4. FLOOR JOIST/ TRUSS MANUFACTURER SHALL COORDINATE LOCATIONS OF ALL MECHANICAL CHASES AND PLUMBING TO AVOID CONFLICT.
5. ALL JOIST TO JOIST OR TRUSS TO TRUSS CONNECTIONS SHALL BE SPECIFIED BY THE MANUFACTURER.
6. G.C./BUILDER SEE ARCHITECTURAL DRAWINGS FOR ROUGH OPENING LOCATIONS AND ADDITIONAL INFORMATION REQ. FOR DOOR AND WINDOW INSTALLATION ALONG WITH DIMENSIONS NOT SHOWN HERE.

FLOOR SHEATHING NOTES

FLOOR SHEATHING TO BE 3/4" PLYWOOD OR OSB (U.N.O), WITH LONG DIMENSION PERPENDICULAR TO SUPPORT (FLOOR TRUSSES).

FLOOR NAILING NOTES

NAILS: NAILS USED IN ALL FLOORING APPLICATIONS SHALL BE 8d, 8d RING OR SPIRAL SHANK, HAND OR GUN DRIVEN NAILS. GUN DRIVEN NAILS SHALL HAVE HEAD SIZE EQUIVALENT TO HAND DRIVEN NAILS, SPACING SHALL BE 6" ON EDGES AND 12" IN FIELD.

NOTES & CONNECTOR LEGEND

1/4"=1'-0"

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COLIN F. GARDELLA, LICENSE No. 73199, STATE OF FLORIDA, PROFESSIONAL ENGINEER

COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

SHEET TITLE
NOTES & CONNECTOR LEGEND

MONTA CONSULTING & DESIGN
OF WMR AND ASSOCIATES, LLC
CERT. OF AUTHORIZATION NO. 9177
1035 GREENWOOD BLVD., SUITE 173
LAKE MARY, FL 32746
PHONE: (407) 681-1917
www.montaconsulting.com

N · C · B · D · C
NATIONAL COUNCIL OF BUILDING DESIGNER CERTIFICATION
Certification No. 10-243

MCD STRENGTH BY DESIGN

PROJECT
COHEN RESIDENCE
1204 N. PARK AVE.
WINTER PARK, FL 32789

STRUCTURAL ENG:	***
DRAFTING:	
-STRUCT:	***
QC CHECK:	***
JOB No.	ZPROP-0206279
SCALE:	AS INDICATED
DATE:	8.26.19

S4.2

WALL LEGEND

- [hatched] = 10'-4" T.O.W. (84'-4")
- [blank] = 10'-6" T.O.W. (84'-6")
- [solid] = INTERIOR/EXTERIOR BEARING WALLS VERIFY HGT. W/ ARCHITECTURAL PLAN U.N.O. OR LINTEL

EXTERIOR FRAME WALL SCHEDULE

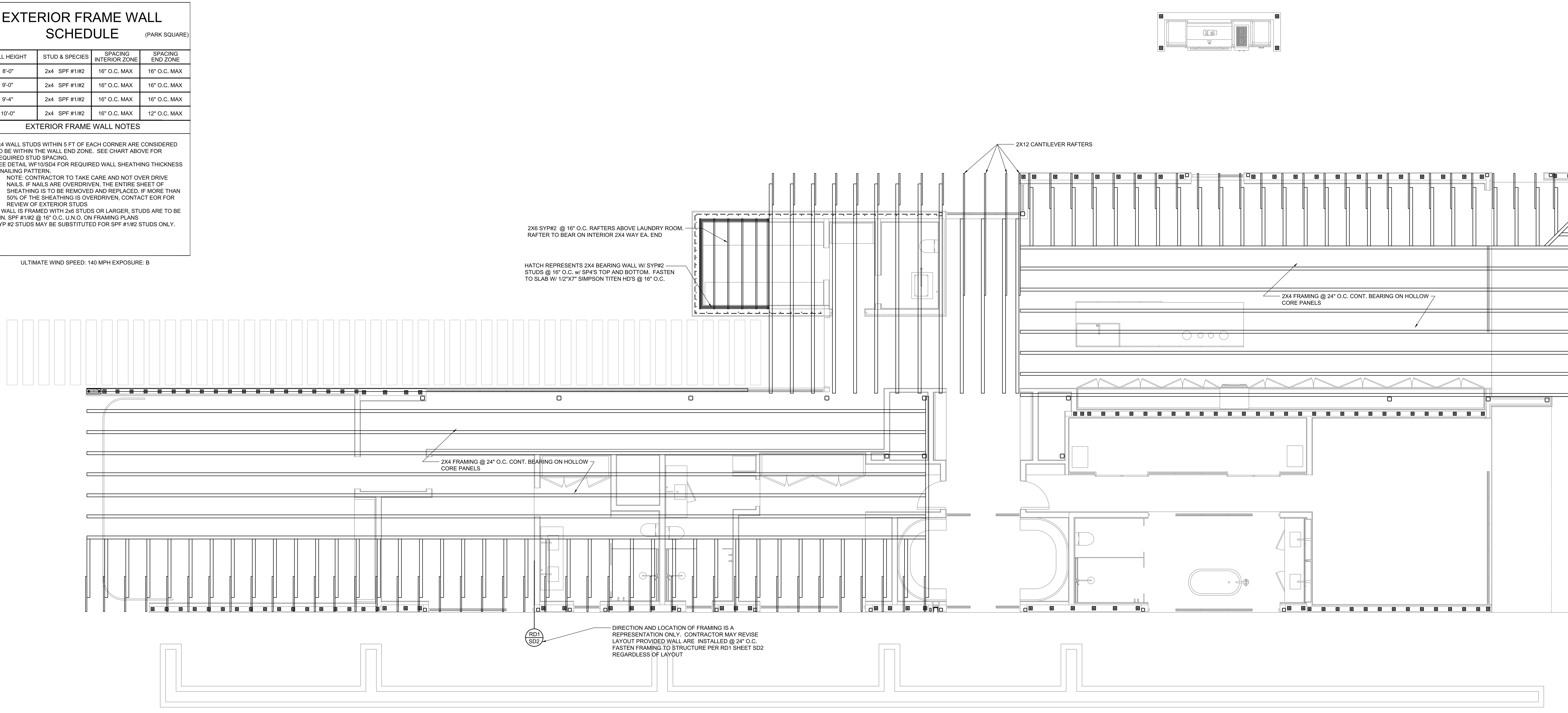
(PARK SQUARE)

WALL HEIGHT	STUD & SPECIES	SPACING INTERIOR ZONE	SPACING END ZONE
8'-0"	2x4 SPF #1/#2	16" O.C. MAX	16" O.C. MAX
9'-0"	2x4 SPF #1/#2	16" O.C. MAX	16" O.C. MAX
9'-4"	2x4 SPF #1/#2	16" O.C. MAX	16" O.C. MAX
10'-0"	2x4 SPF #1/#2	16" O.C. MAX	12" O.C. MAX

EXTERIOR FRAME WALL NOTES

1. 2x4 WALL STUDS WITHIN 5 FT OF EACH CORNER ARE CONSIDERED TO BE WITHIN THE WALL END ZONE. SEE CHART ABOVE FOR REQUIRED STUD SPACING.
2. SEE DETAIL WF10/SD4 FOR REQUIRED WALL SHEATHING THICKNESS & NAILING PATTERN.
   2.1. NOTE: CONTRACTOR TO TAKE CARE AND NOT OVER DRIVE NAILS. IF NAILS ARE OVERDRIVEN, THE ENTIRE SHEET OF SHEATHING IS TO BE REMOVED AND REPLACED. IF MORE THAN 50% OF THE SHEATHING IS OVERDRIVEN, CONTACT EOR FOR REVIEW OF EXTERIOR STUDS
3. IF WALL IS FRAMED WITH 2x6 STUDS OR LARGER, STUDS ARE TO BE MIN. SPF #1/#2 @ 16" O.C. U.N.O. ON FRAMING PLANS
4. SYP #2 STUDS MAY BE SUBSTITUTED FOR SPF #1/#2 STUDS ONLY.

ULTIMATE WIND SPEED: 140 MPH EXPOSURE: B



NOTE: SEE PAGE S4.2 FOR NOTES AND CONNECTOR LEGEND

ROOF FRAMING PLAN

1/4"=1'-0"

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FLORIDA LIC. No. 73199

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ROOF FRAMING PLAN

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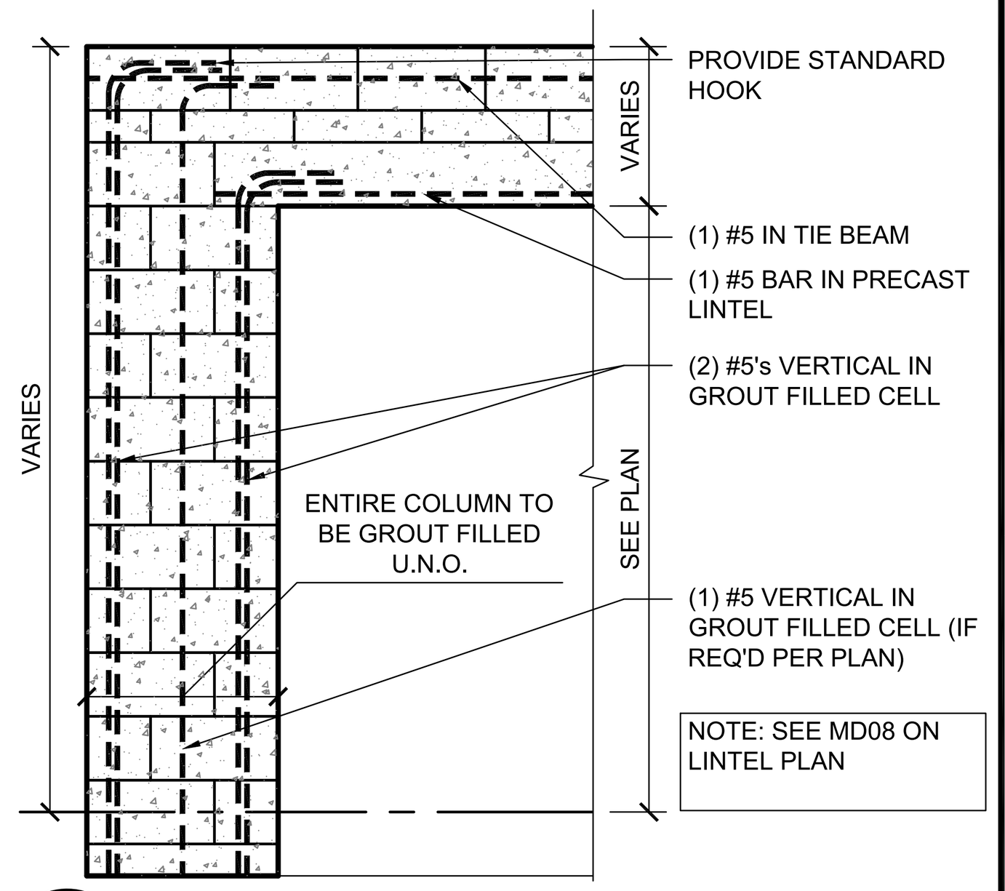
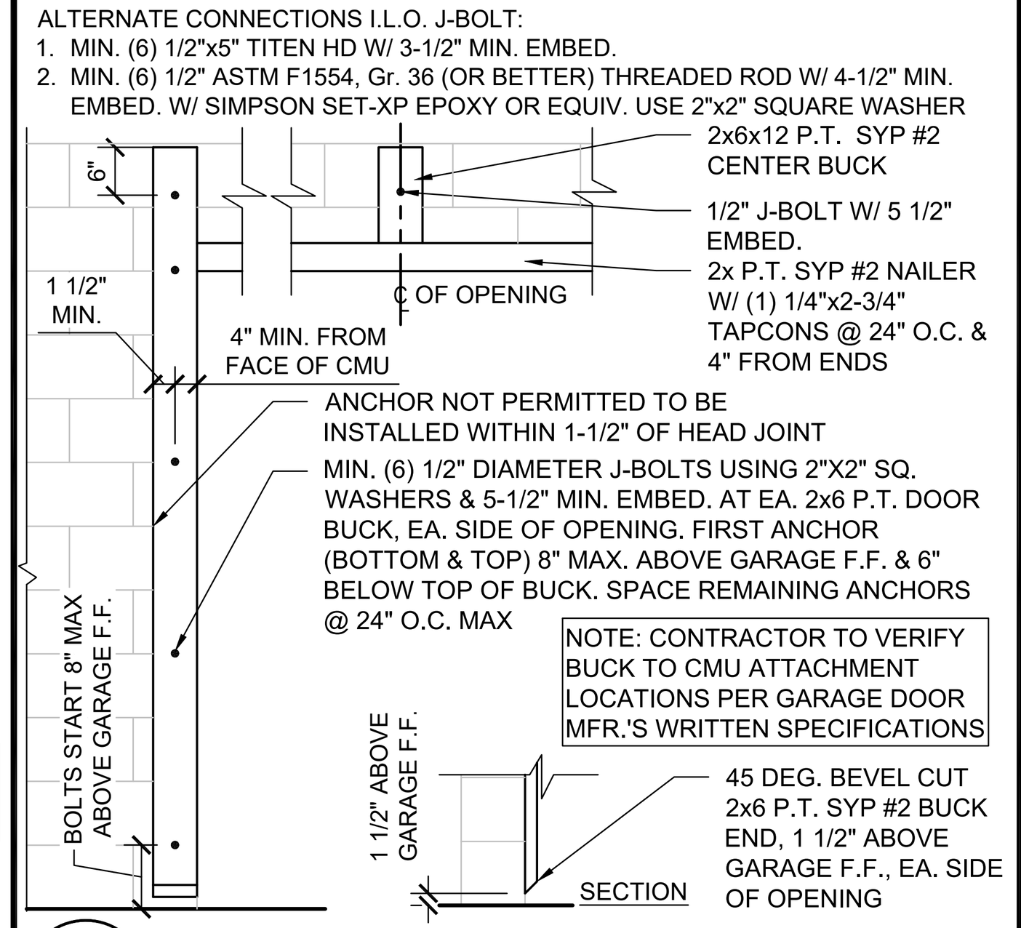
S4.3



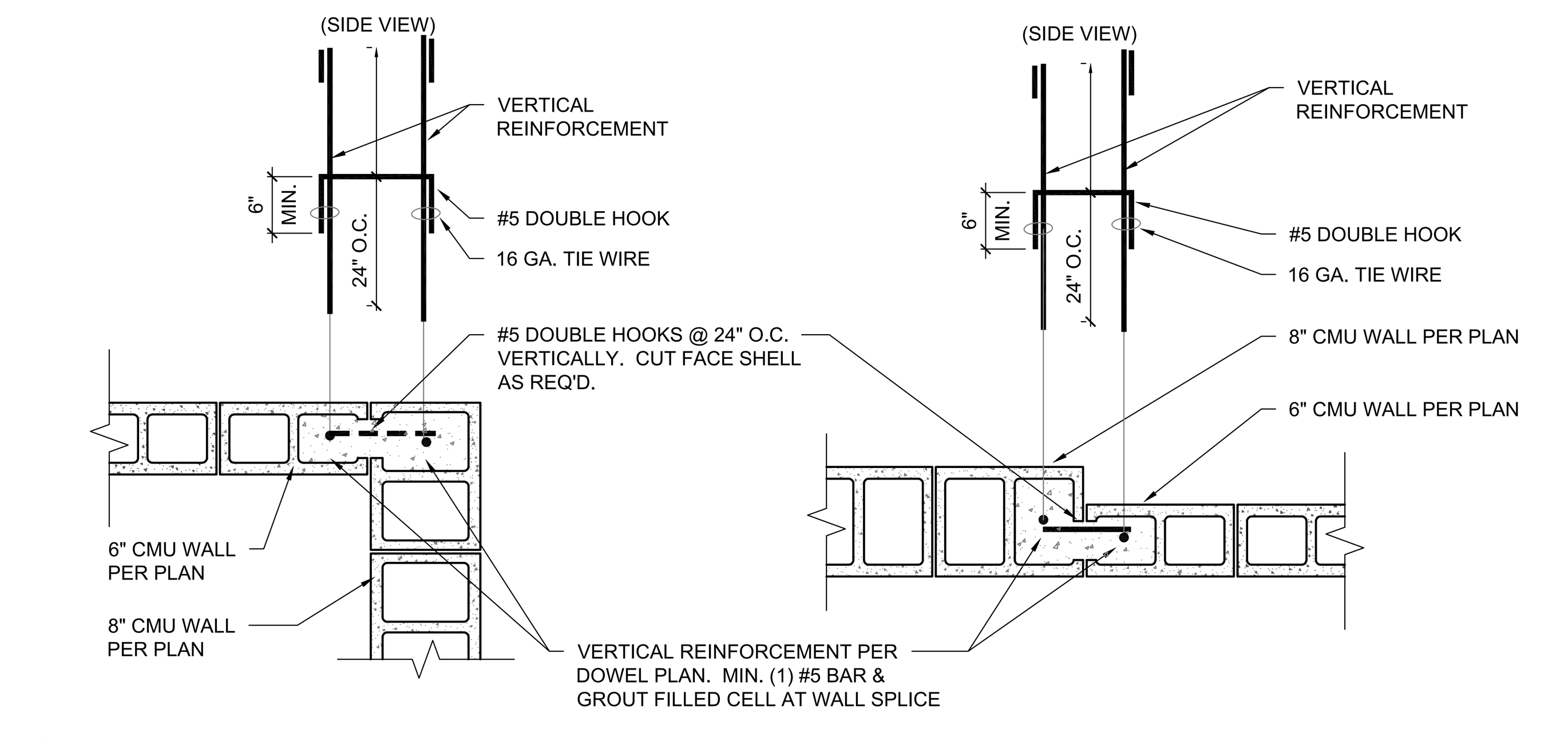
MD01 VERTICAL REINFORCEMENT DETAIL

1/2"=1'-0"



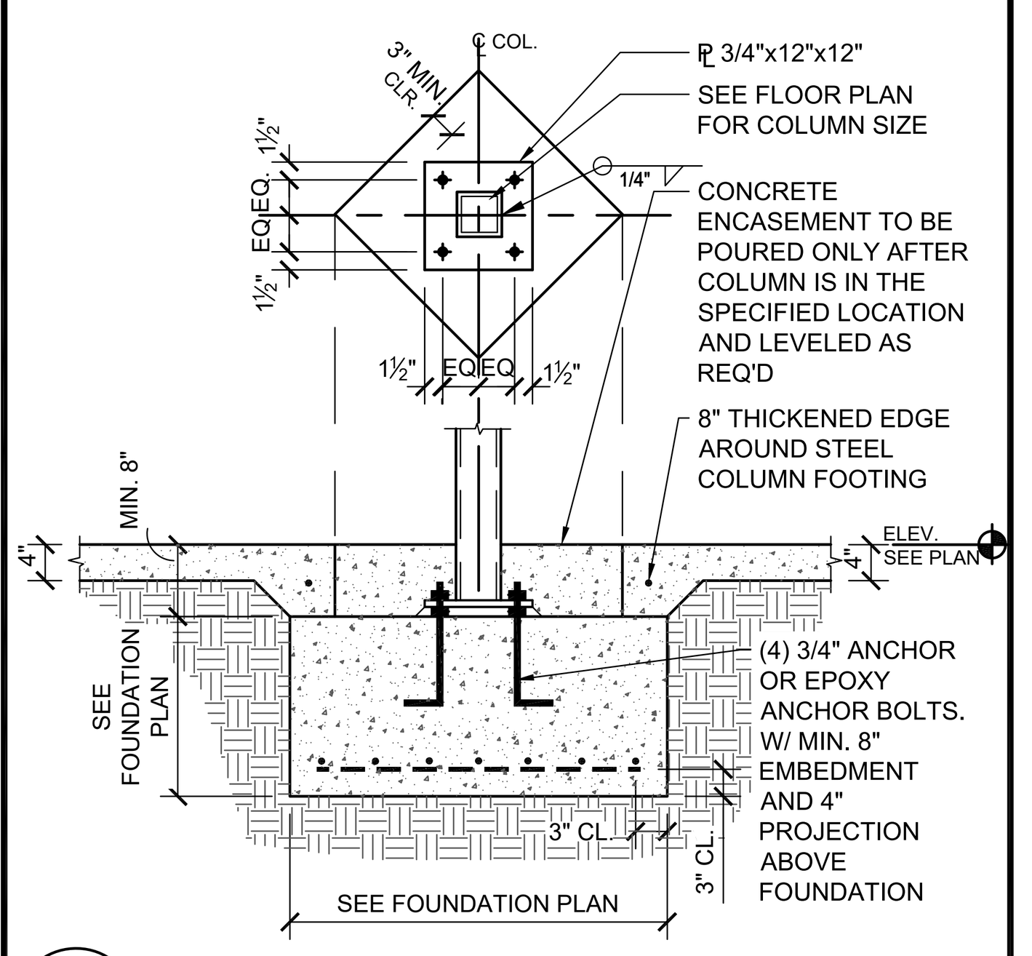
MD02 GARAGE BUCK DETAIL (MAX. 170C)

1/2"=1'-0"



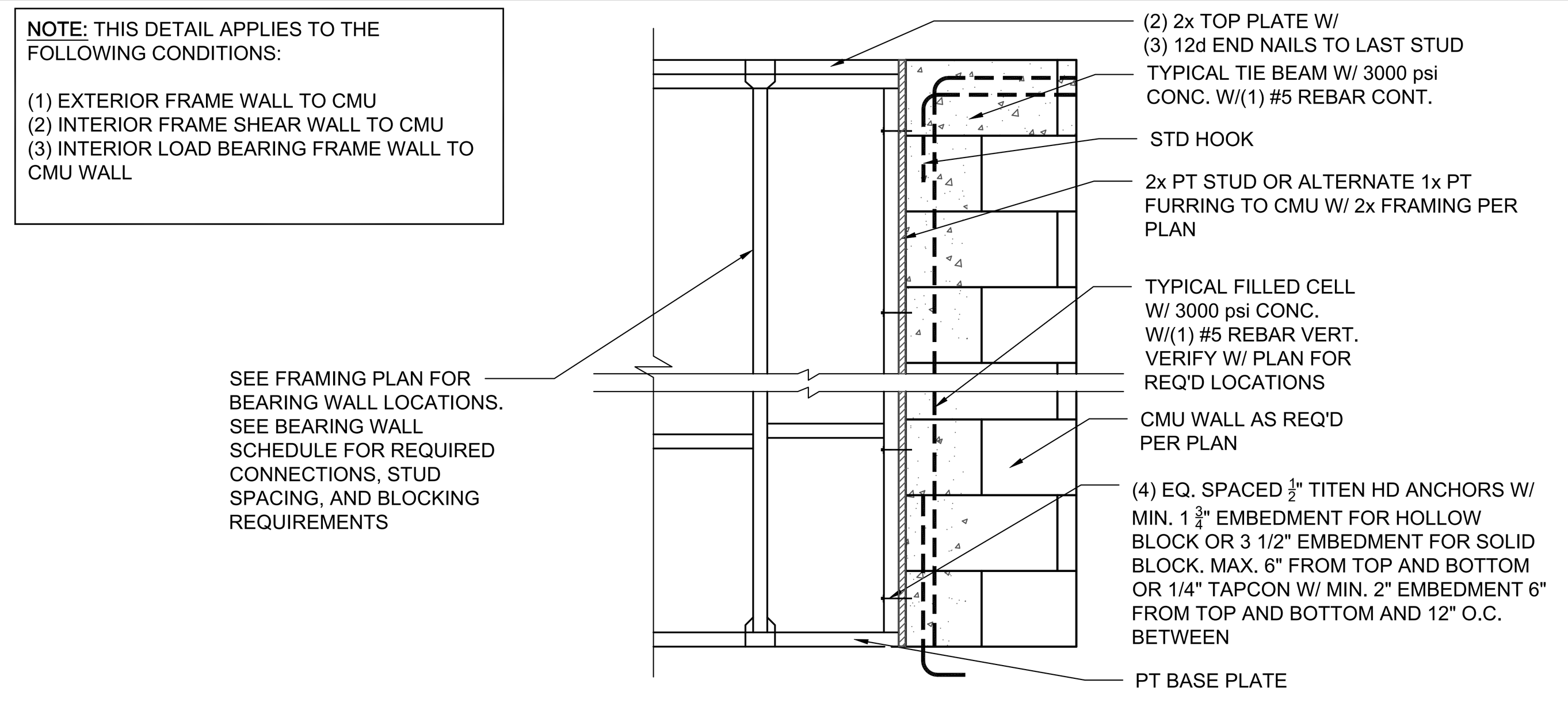
MD25 TYPICAL 6 TO 8 CMU SPLICE W REBAR TIE

3/4"=1'-0"



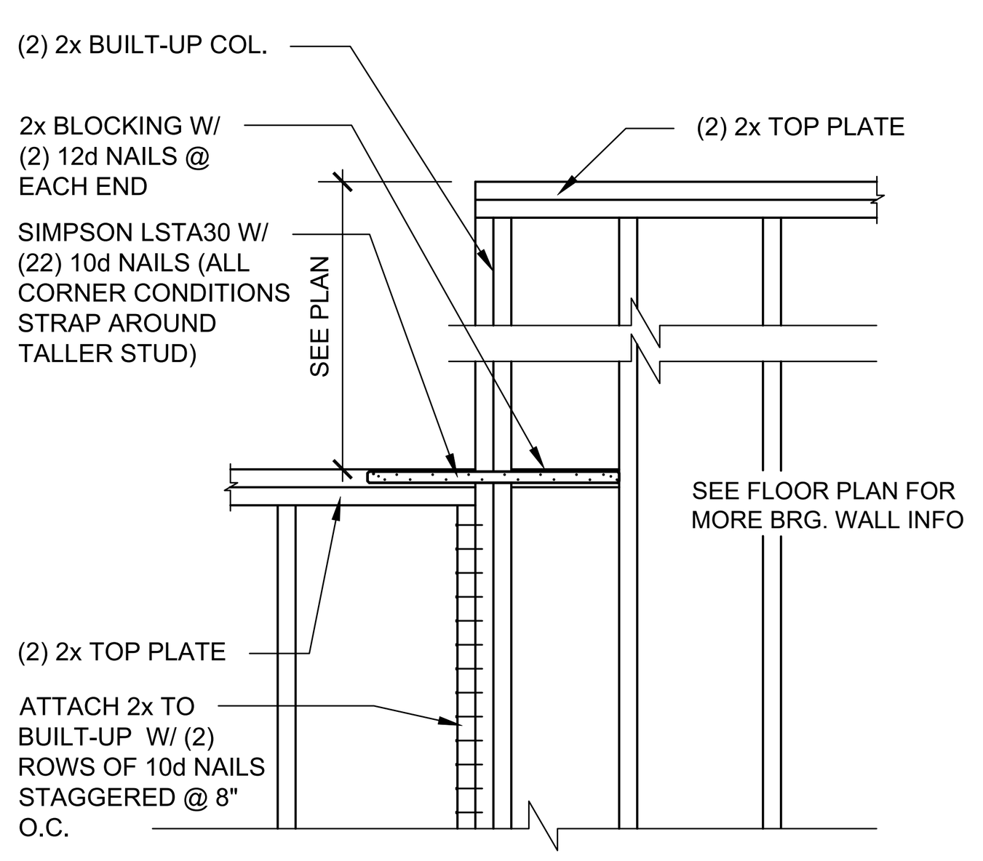
ST04 STEEL COLUMN @ FOUNDATION

3/4"=1'-0"



MD03 BEARING/ SHEARWALL CONNECTION TO CMU

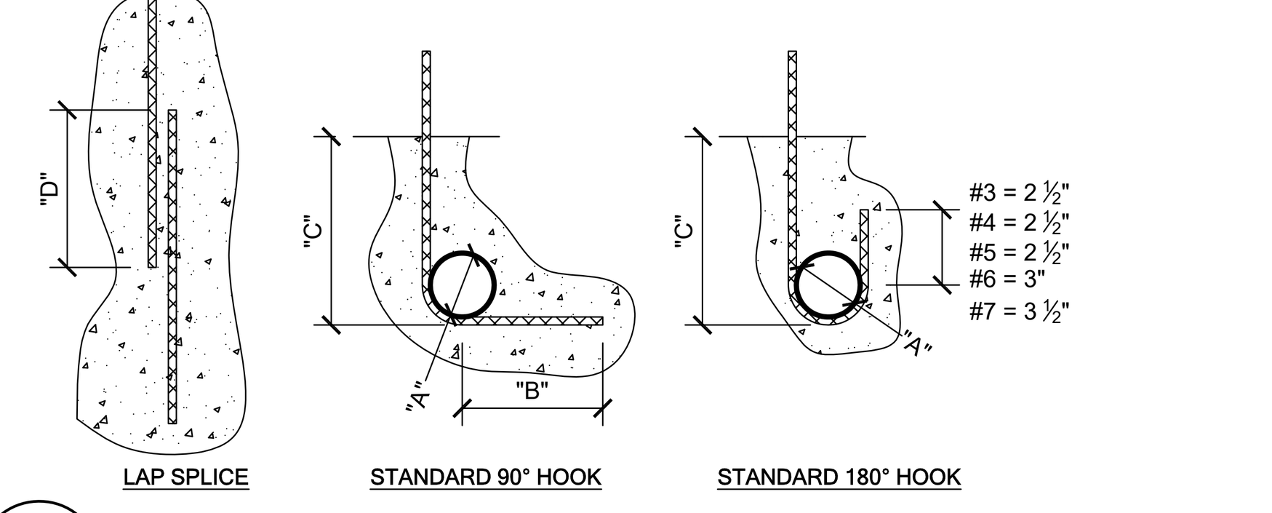
3/4"=1'-0"



WF18 STEP @ BEARING FRAME WALL

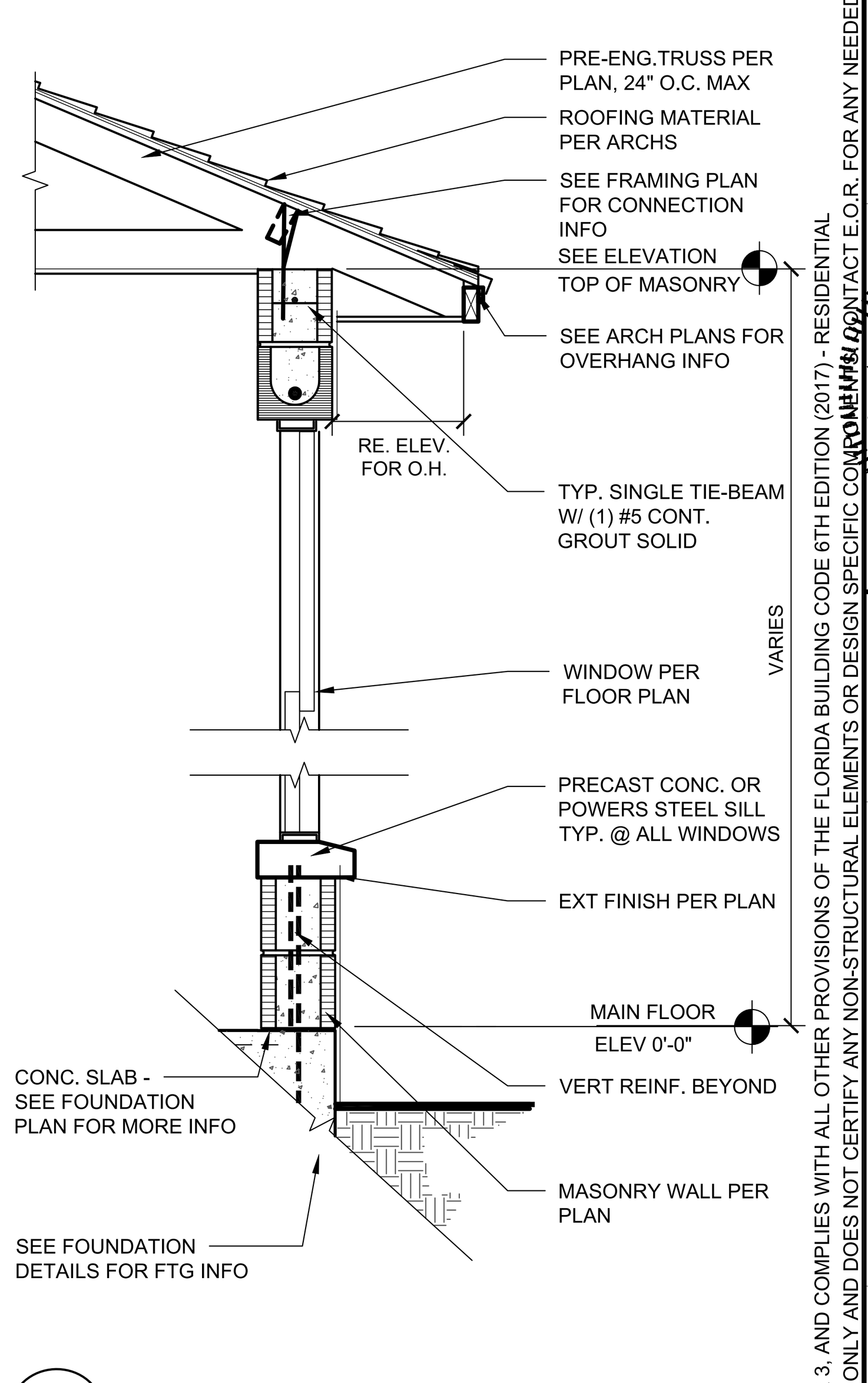
3/4"=1'-0"

REQ'D LAP SPLICE, EMBEDMENT, AND HOOKED REINF. STEEL (INCHES)									
	GRADE 40				GRADE 60				
BAR SIZE	BEND Ø	LENGTH		LAP SPLICE	BEND Ø	LENGTH		LAP SPLICE	
	"A"	"B"	"C"	"D"	"A"	"B"	"C"	"D" MASONRY	"D" CONCRETE
#3	2-1/4"	4-1/2"	6"	15"	2-1/4"	4-1/2"	6"	15"	18"
#4	3"	6"	6"	20"	3"	6"	6"	20"	24"
#5	3-3/4"	7-1/2"	6"	25"	3-3/4"	7-1/2"	6"	25"	30"
#6	4-1/2"	9"	6"	30"	4-1/2"	9"	6"	40"	36"
#7	5-1/4"	10-1/2"	7"	35"	5-1/4"	10-1/2"	7"	53"	53"



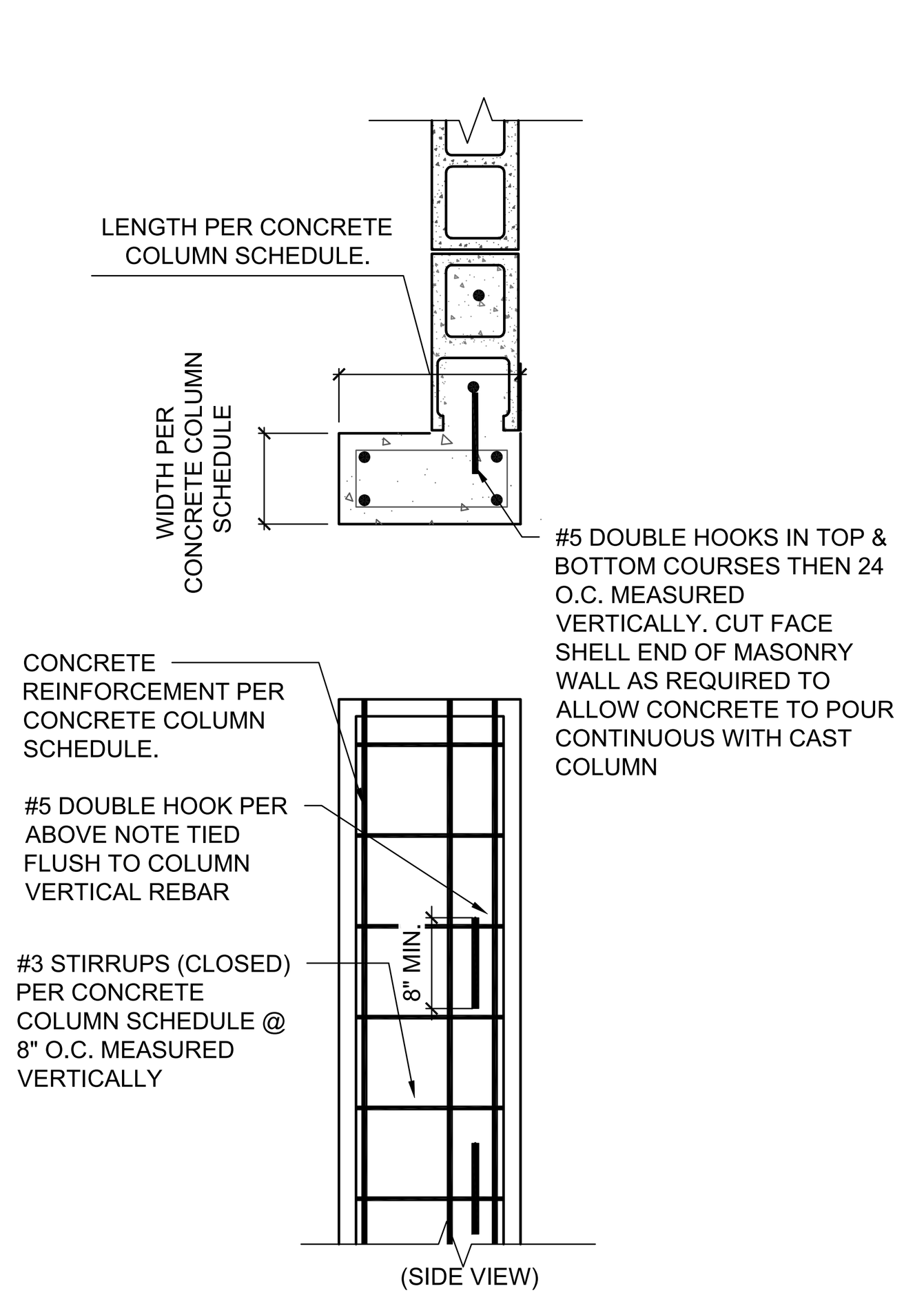
MD08 REBAR SPLICE DETAIL

3/4"=1'-0"



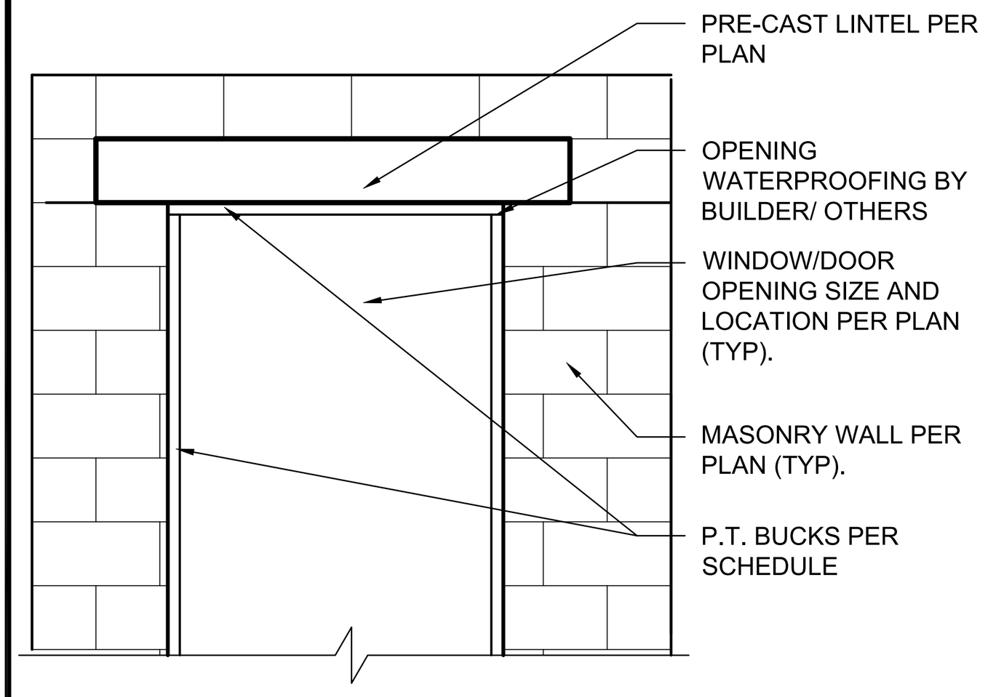
WS01 TYPICAL ONE STORY CMU WALL SECTION

3/4"=1'-0"



CC05 CMU TO CAST IN PLACE TIE IN

3/4"=1'-0"



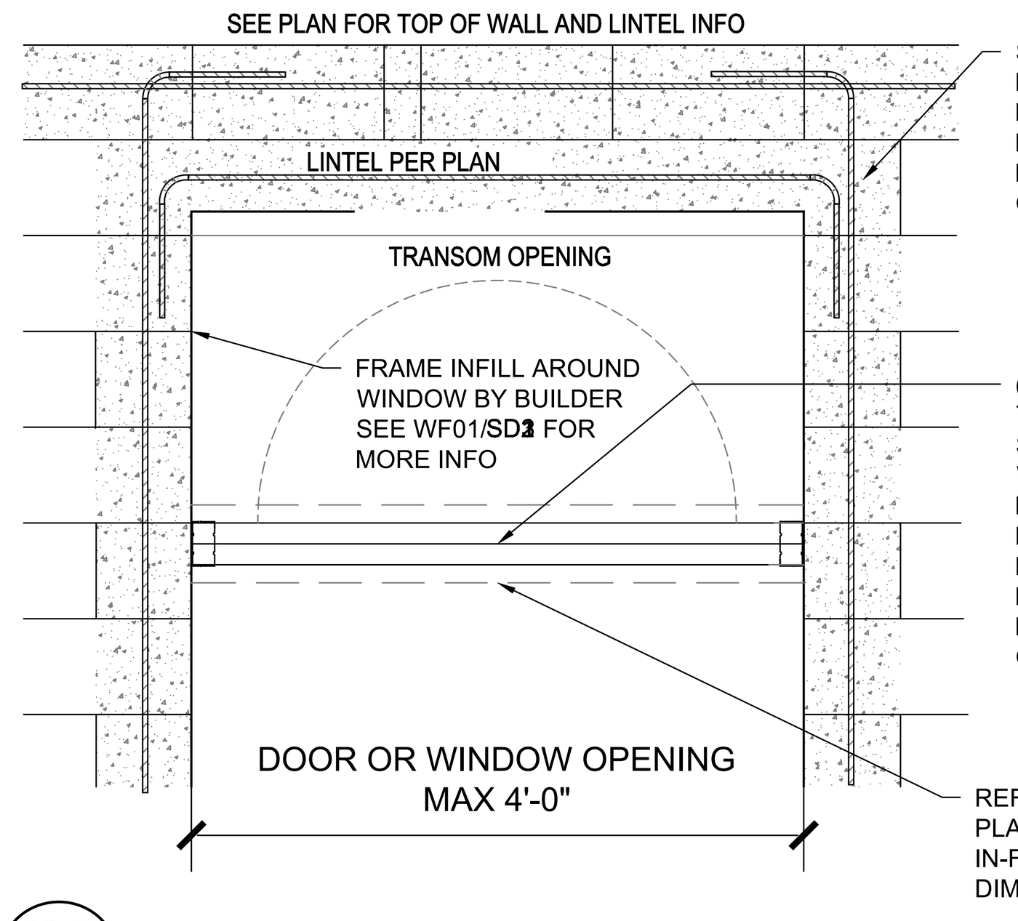
P.T. BUCK FASTENER SCHEDULE			
SIZE	TAPCONS	SPACING	# ROWS
1x4	3/16" x 2 1/4"	12" O.C.	1
1x6	3/16" x 2 1/4"	12" O.C.	2
1x8	3/16" x 2 1/4"	12" O.C.	2
2x4	3/16" x 3 1/4"	12" O.C.	1
2x6	3/16" x 3 1/4"	12" O.C.	2
2x8	3/16" x 3 1/4"	12" O.C.	2

NOTES:

1. 2x SYP #2 P.T. BUCKS/NAILERS MAY EXTEND BEYOND INT. FACE OF WINDOW/DOOR. FASTEN BUCK TO MASONRY PER FASTENER SCHEDULE 4" FROM ENDS AND 12" O.C. MAX. TAPCONS TO BE 1 1/2" MIN. FROM EDGE OF BUCK.
2. BUCKS LESS THAN 2x SYP #2 TO BE FASTENED W/CUT NAILS OR EQUIVALENT, STRUCTURAL CONNECTION OF WINDOW/DOOR TO STRUCTURE BY OTHERS IN THIS CASE.
3. ADDITIONAL 2x SYP #2 P.T. WOOD BUCK MATERIAL MAY BE ADDED TO EXISTING 2x BUCKED OPENING, USE (3) ROWS OF 10d x 3" NAILS STAGGERED @ 12" O.C. AND 6" FROM ENDS @ EACH SUCCESSIVE BUCK. MAX. 2 PLIES OVER INITIAL P.T. BUCK.
4. MIXING OF 1x AND 2x BUCKS SHALL BE PERMITTED FOR THE SAME OPENING IF WINDOW/DOOR MANUFACTURER AND PLAN REQUIREMENTS (FASTENING) ARE MET.
5. MAINTAIN MAXIMUM 2" FASTENER EMBEDMENT INTO HOLLOW OR SOLID GROUTED CMU.
6. WINDOW OR DOOR INSTALLATION BY OTHERS

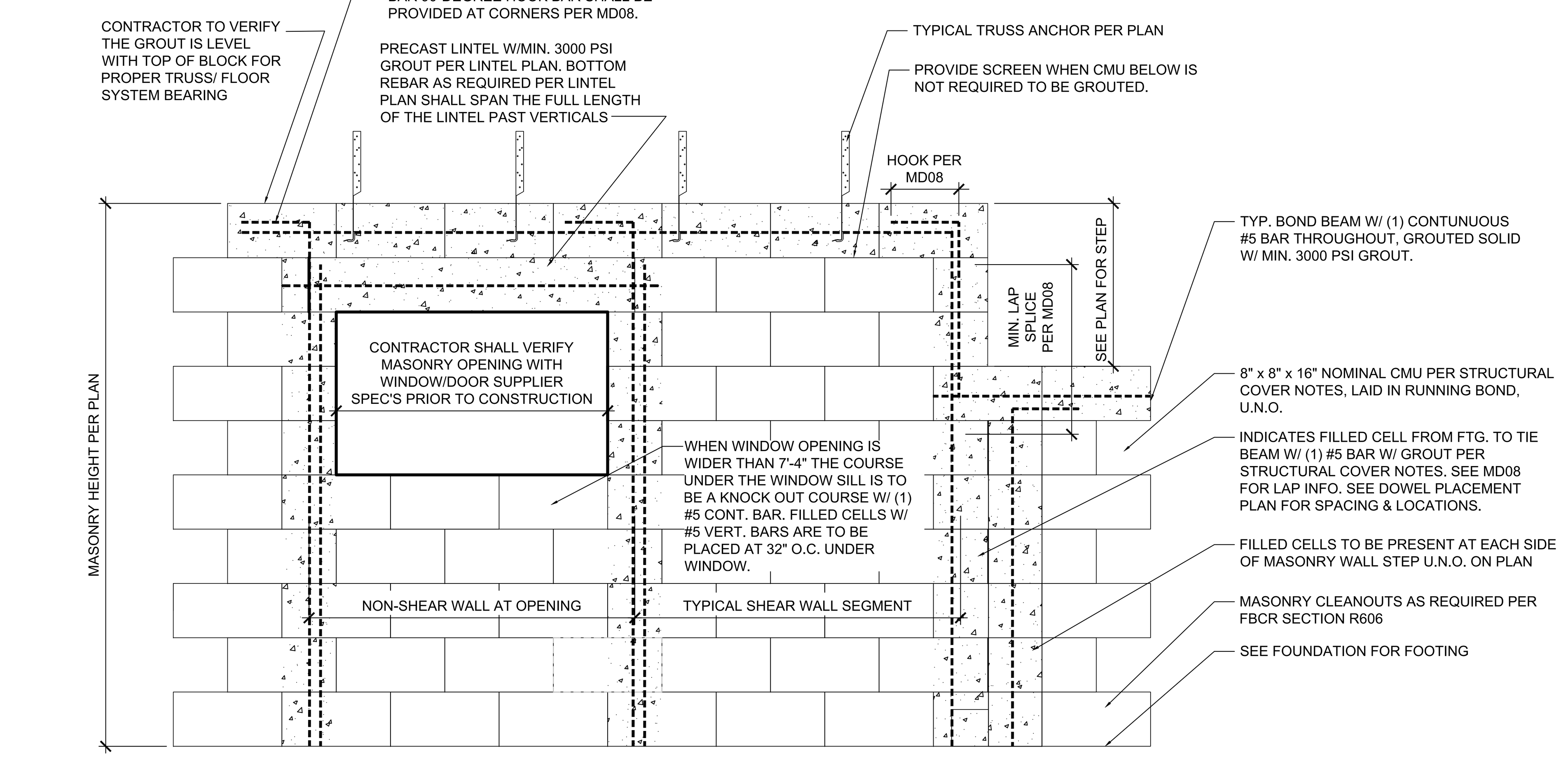
MD04 2x BUCK DETAIL

1/2"=1'-0"



MD07 LINTEL W/ TRANSOM DETAIL

3/4"=1'-0"



MD06 TYPICAL MASONRY CONSTRUCTION

3/4"=1'-0"

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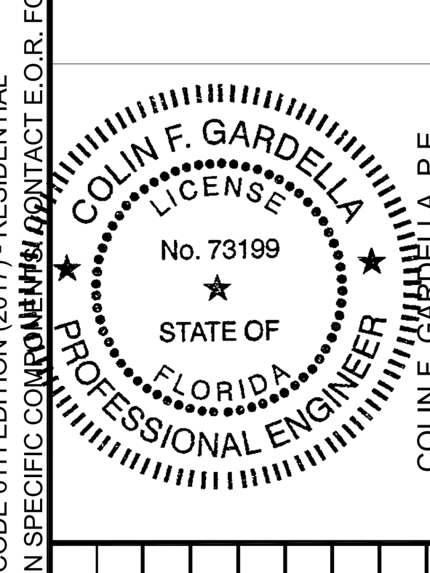

COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

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GENERAL DETAILS

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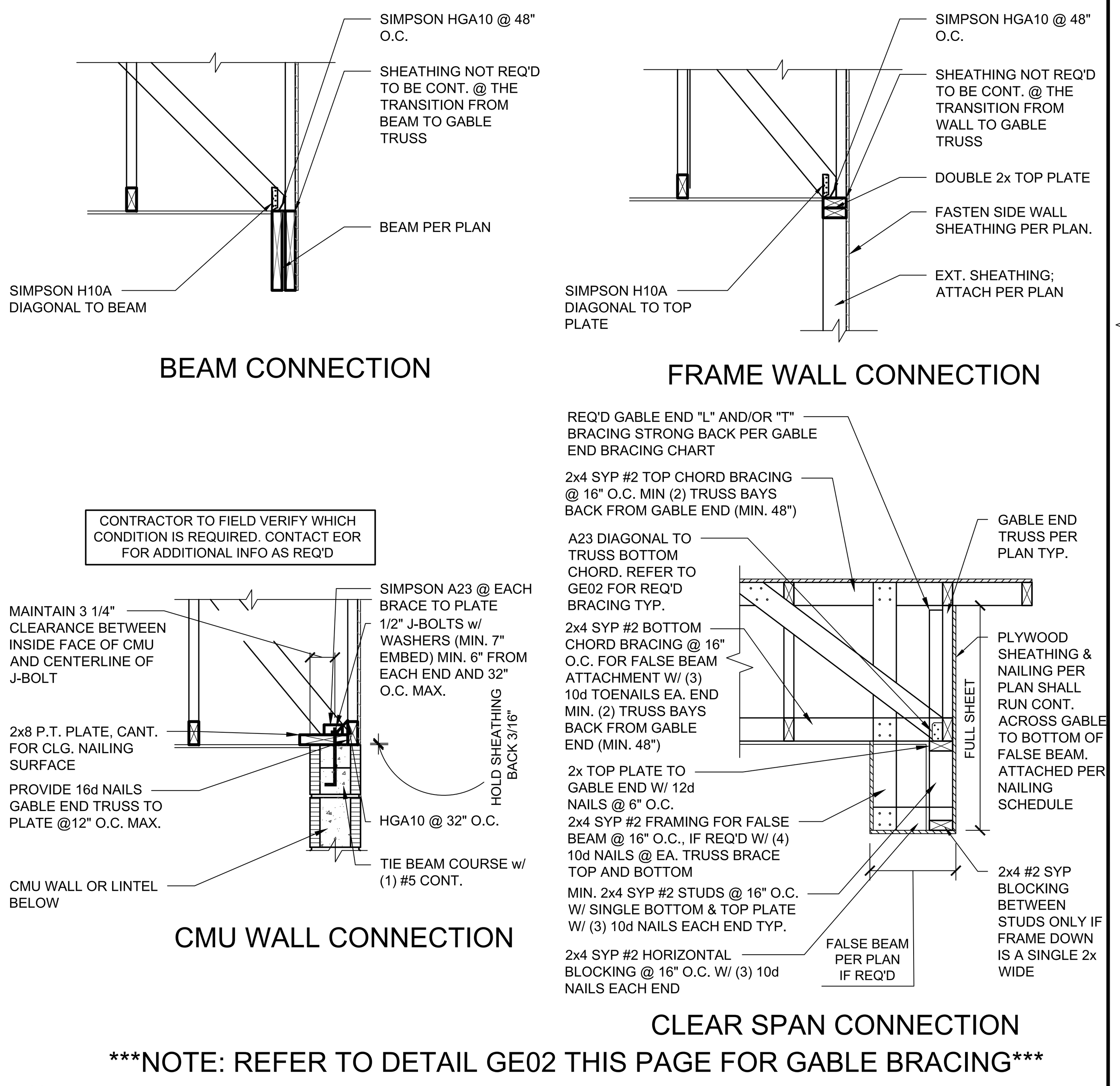
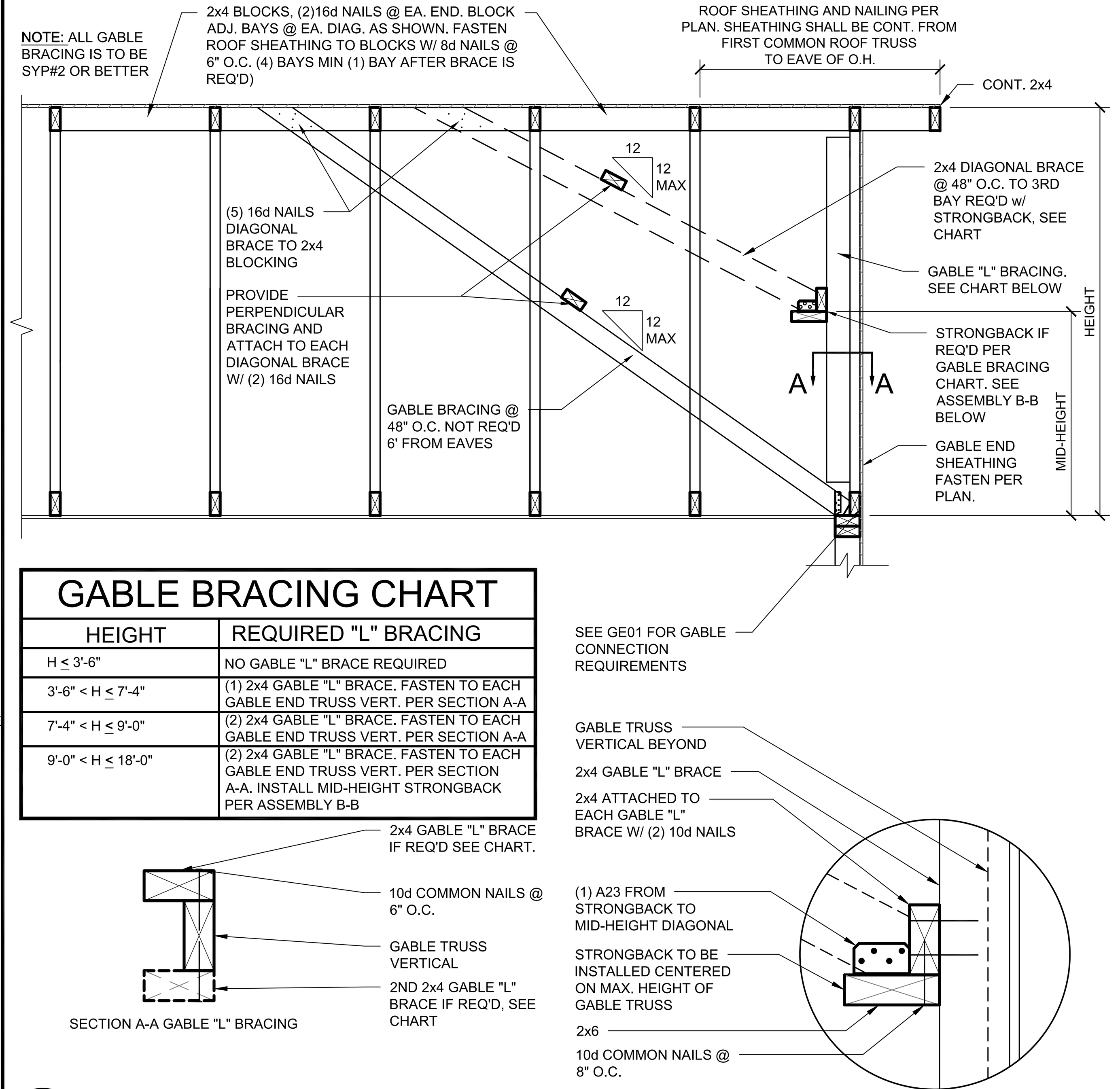
SD1



GE01 GABLE END @ FLAT CEILING CONNECTIONS

3/4"=1'-0"
(150 MPH EXP. C MAX)



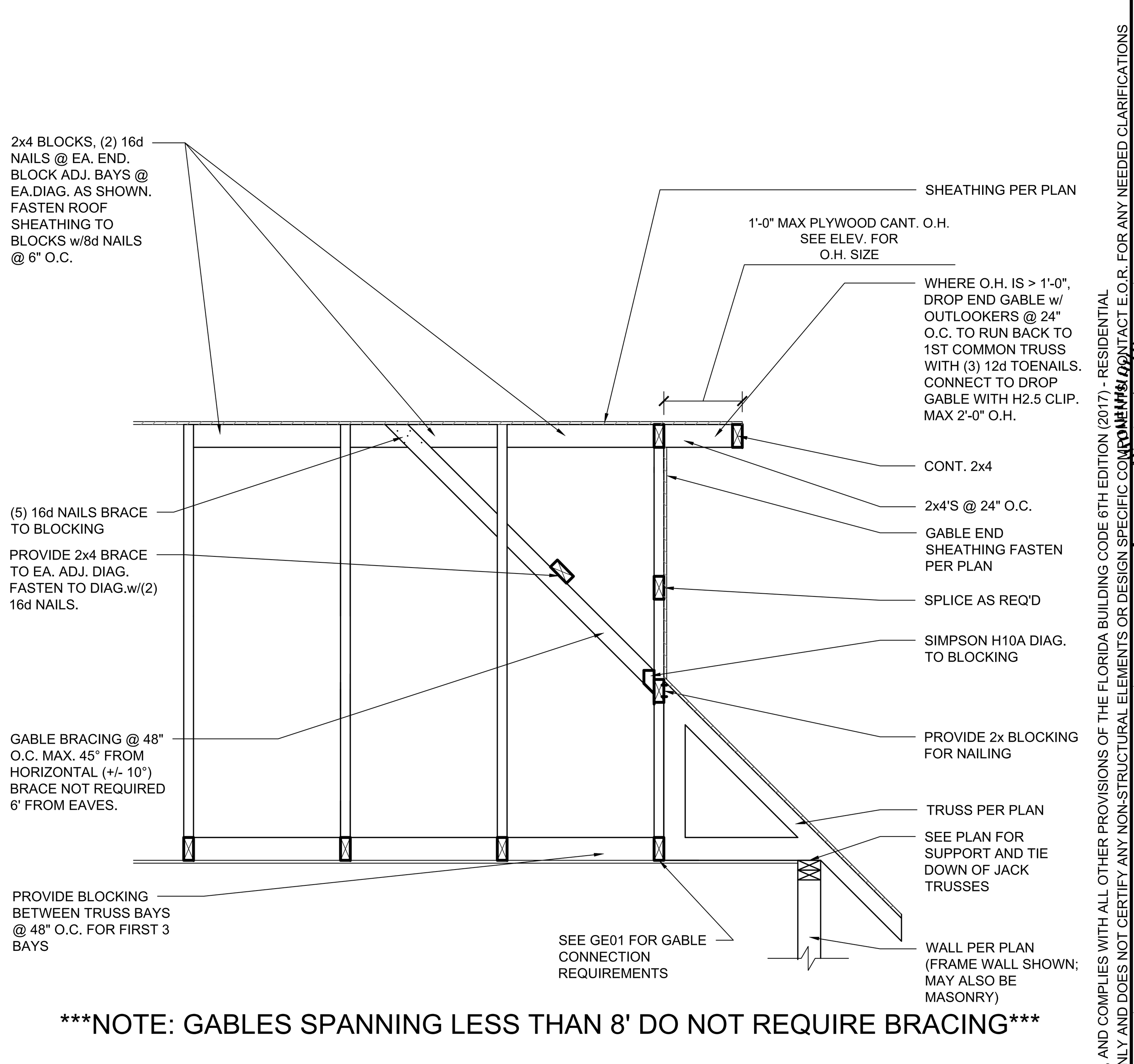
GABLE BRACING CHART

HEIGHT	REQUIRED "L" BRACING
H ≤ 3'-6"	NO GABLE "L" BRACE REQUIRED
3'-6" < H ≤ 7'-4"	(1) 2x4 GABLE "L" BRACE. FASTEN TO EACH GABLE END TRUSS VERT. PER SECTION A-A
7'-4" < H ≤ 9'-0"	(2) 2x4 GABLE "L" BRACE. FASTEN TO EACH GABLE END TRUSS VERT. PER SECTION A-A
9'-0" < H ≤ 18'-0"	(2) 2x4 GABLE "L" BRACE. FASTEN TO EACH GABLE END TRUSS VERT. PER SECTION A-A. INSTALL MID-HEIGHT STRONGBACK PER ASSEMBLY B-B

GE02 GABLE END BRACING

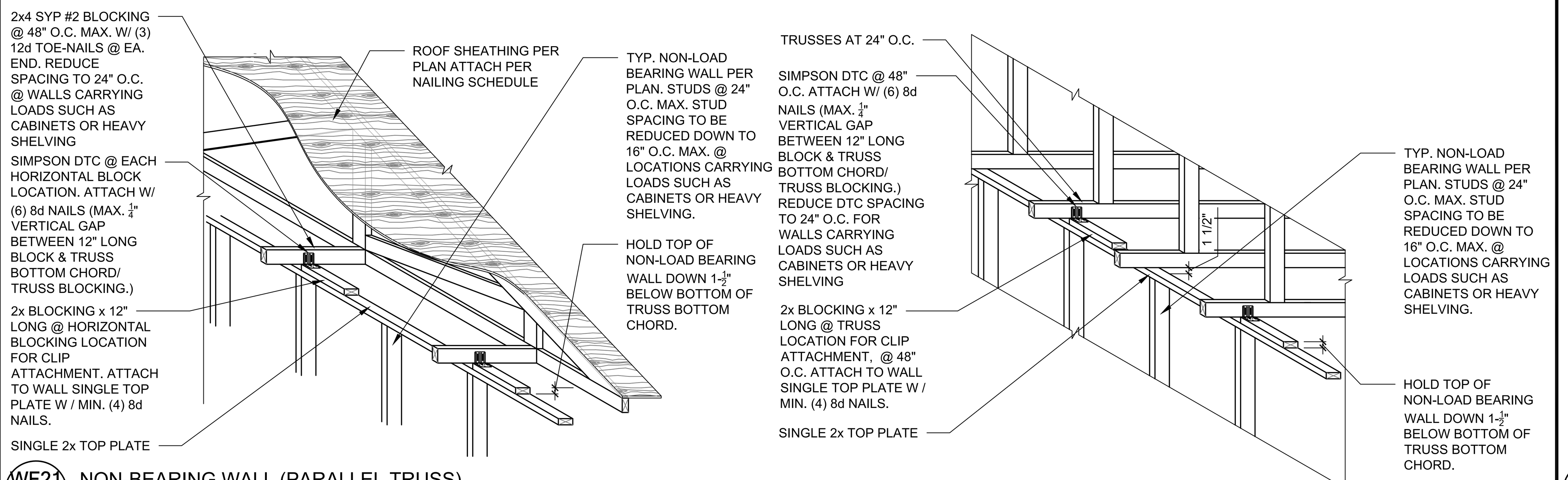
WIND SPEED & EXPOSURE: 140C MAX

3/4"=1'-0"



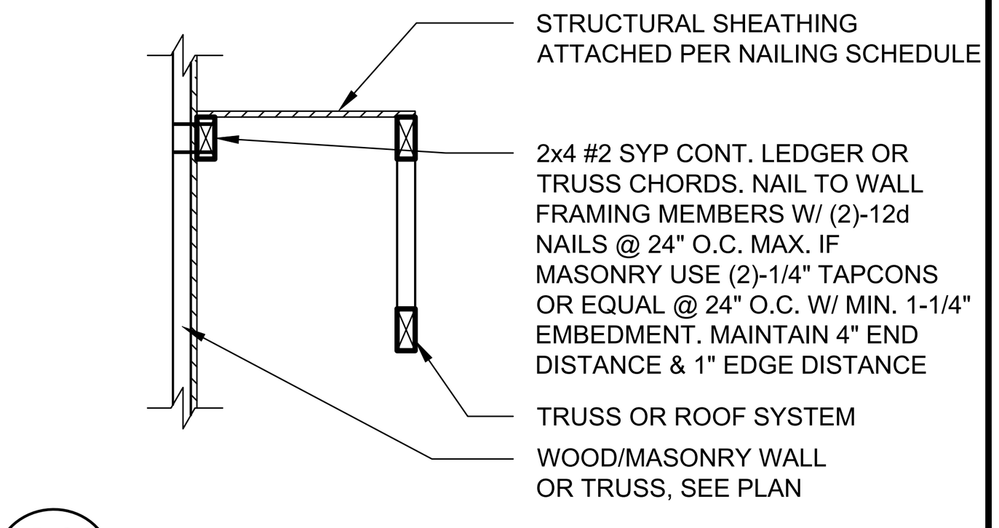
GE03 GABLE END @ FLAT CEILING W/ BOSTON HIP

3/4"=1'-0"



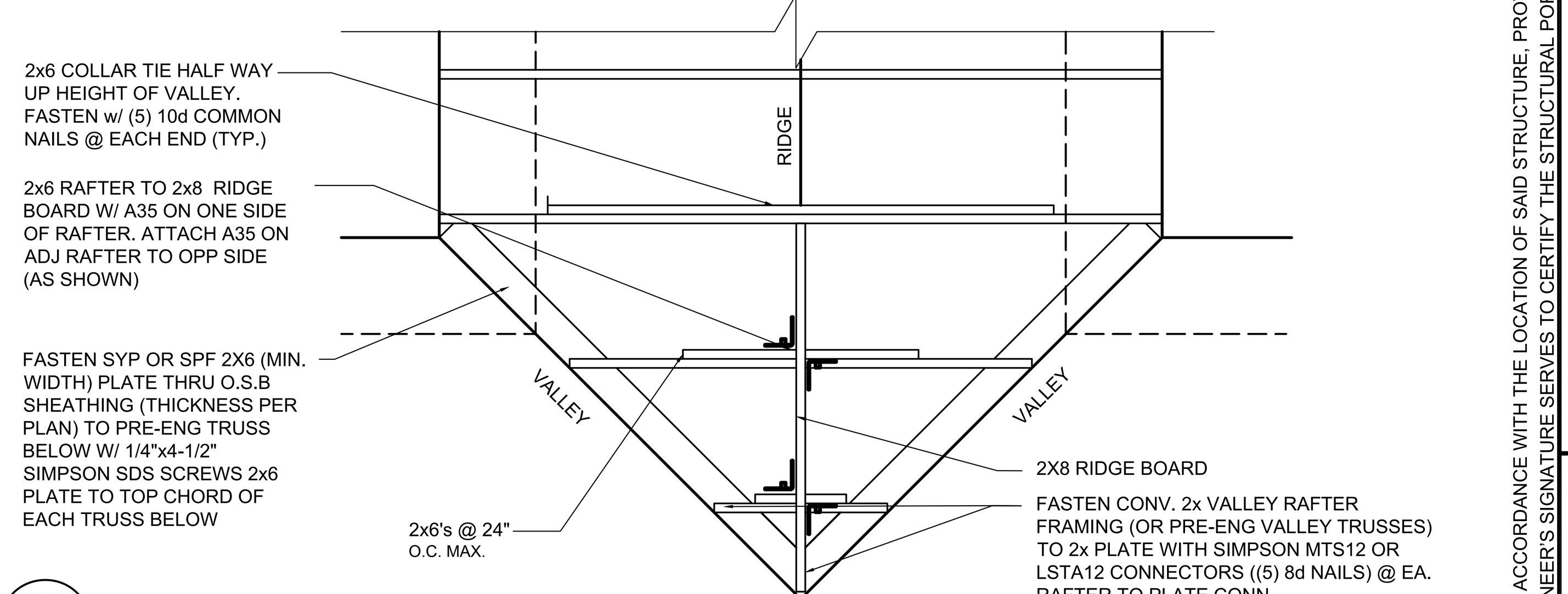
WF21 NON-BEARING WALL (PARALLEL TRUSS)

1/2"=1'-0"



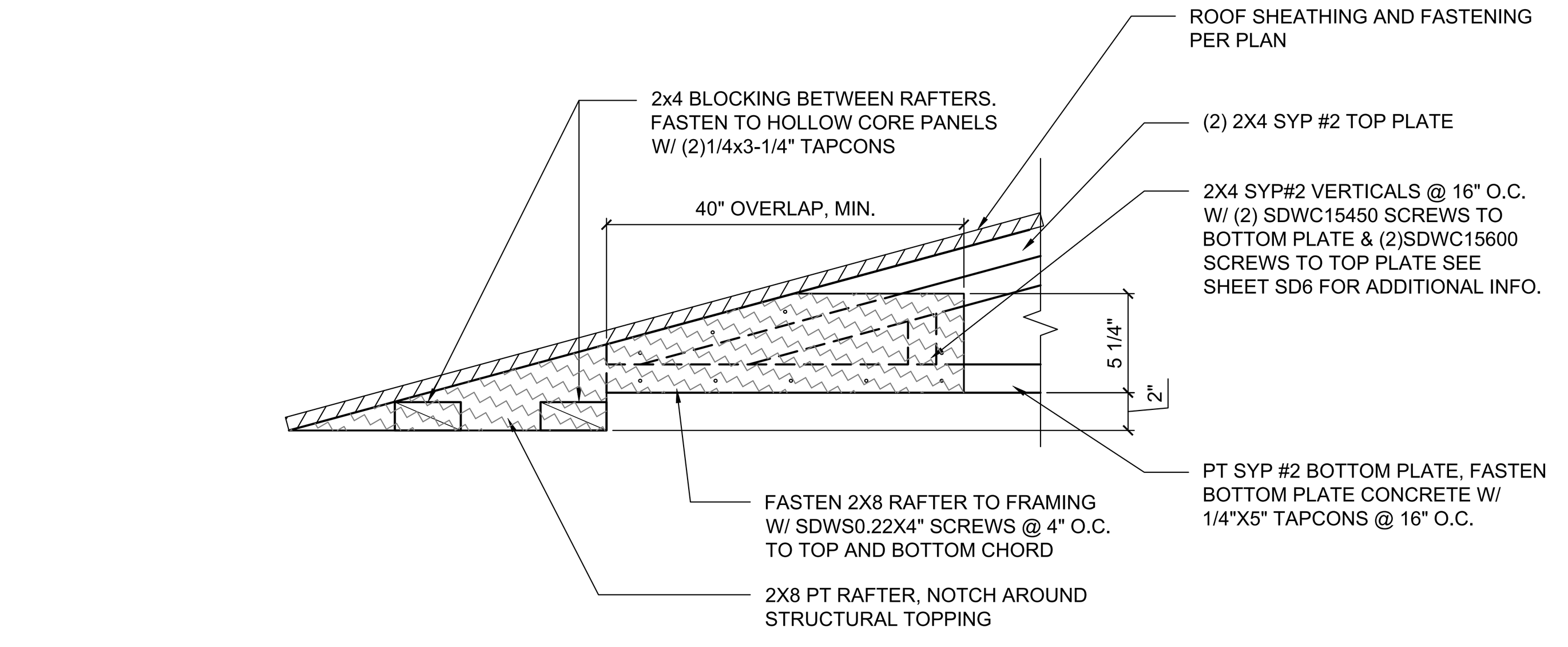
LD02 LEDGER @ LOW ROOF

3/4"=1'-0"



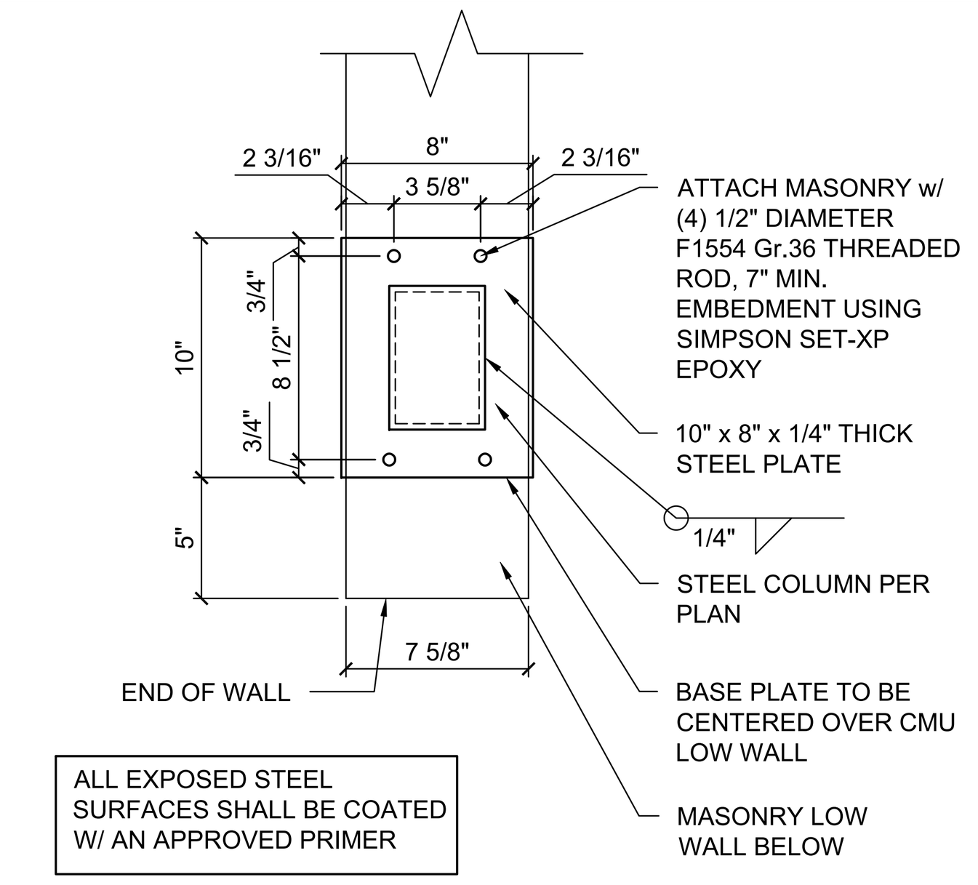
CF01 FIELD FRAMED VALLEY DETAIL

1/2"=1'-0"



RD1 NOTCHED RAFTER AND FRAMING ATTACHMENT DETAIL

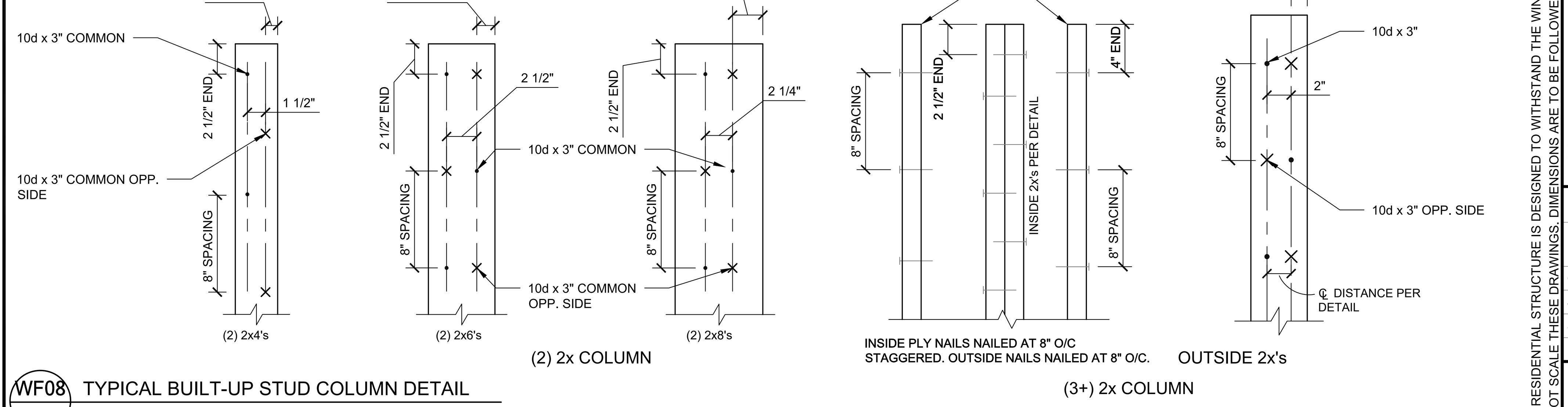
1 1/2"=1'-0"



SC6 STEEL COLUMN BASE CONNECTION

1 1/2"=1'-0"

NOTES:
1) ADJACENT NAILS ARE DRIVEN FROM OPPOSITE SIDES OF THE COLUMN
2) ALL NAILS PENETRATE AT LEAST 3/4" OF THE THICKNESS OF THE LAST LAMINATION
3) FOR 5-PLY, PROVIDE 1/4"DIA. x 5 1/2" LAG SCREWS OR EQUAL (SPACE AS SHOWN FOR 3-PLY)
4) TOLERANCES: ALL DIMENSION TO BE HELD WITHIN +/- 1/4"



WF08 TYPICAL BUILT-UP STUD COLUMN DETAIL

1 1/2"=1'-0"

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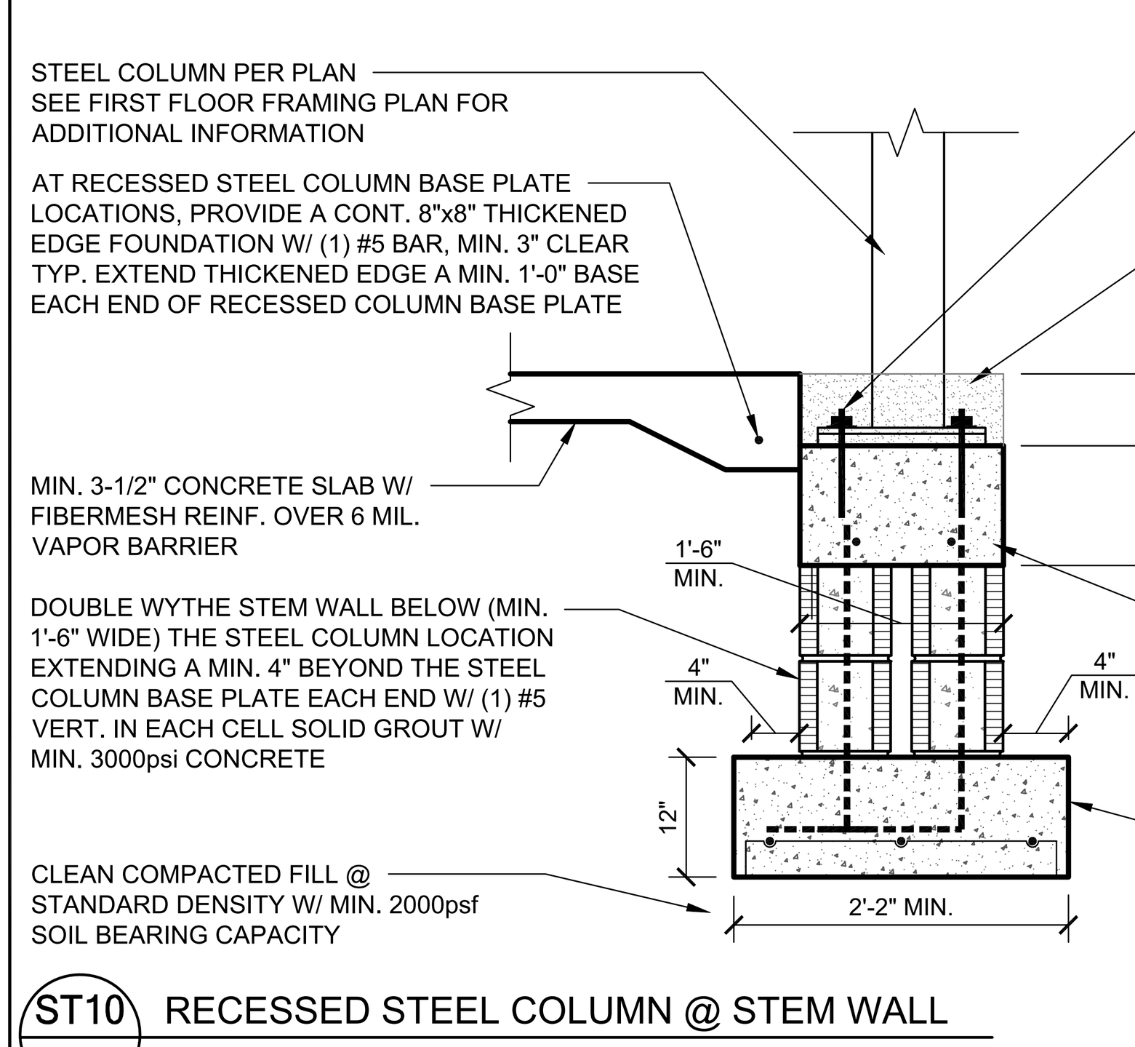
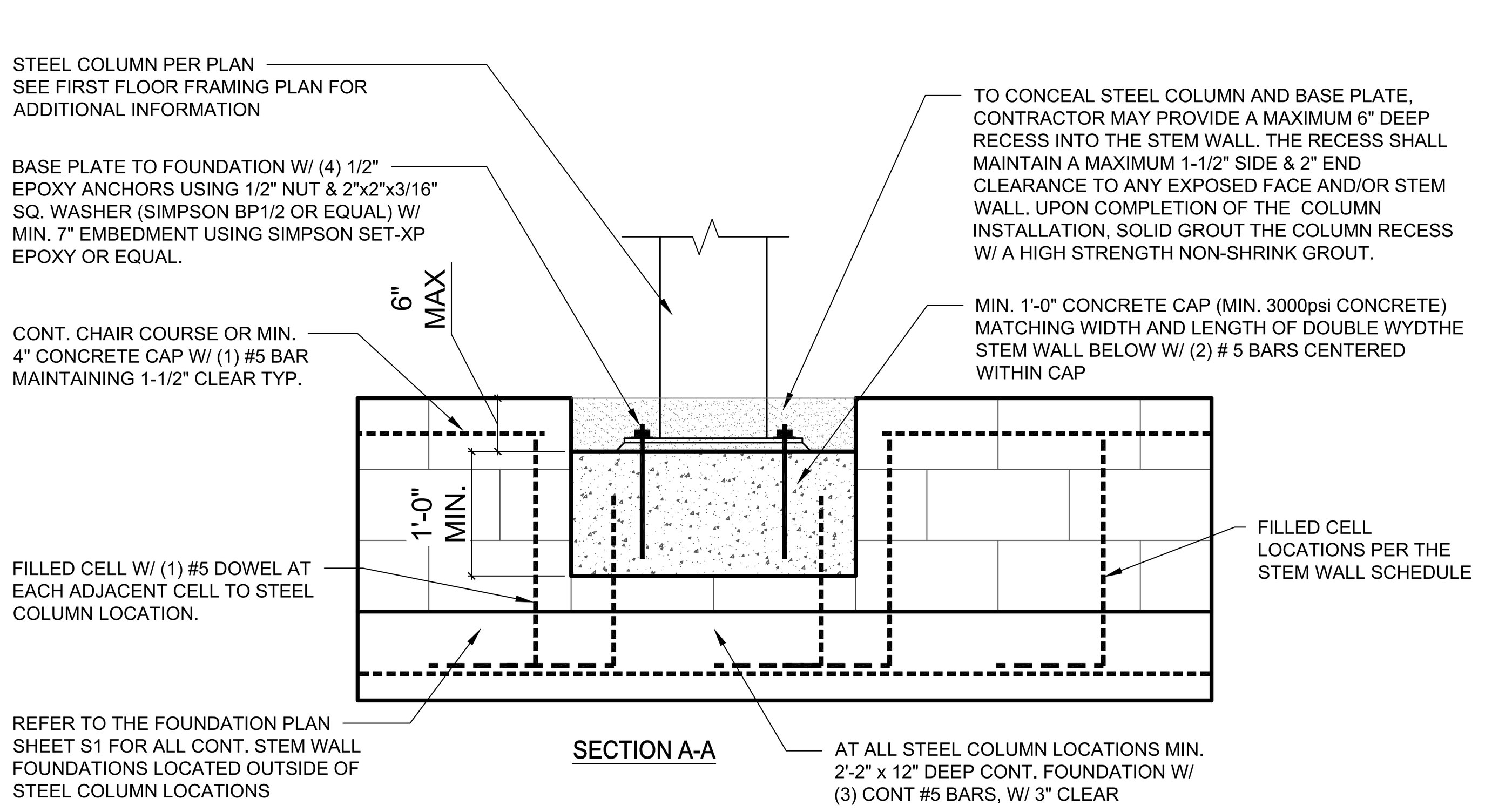
SD2

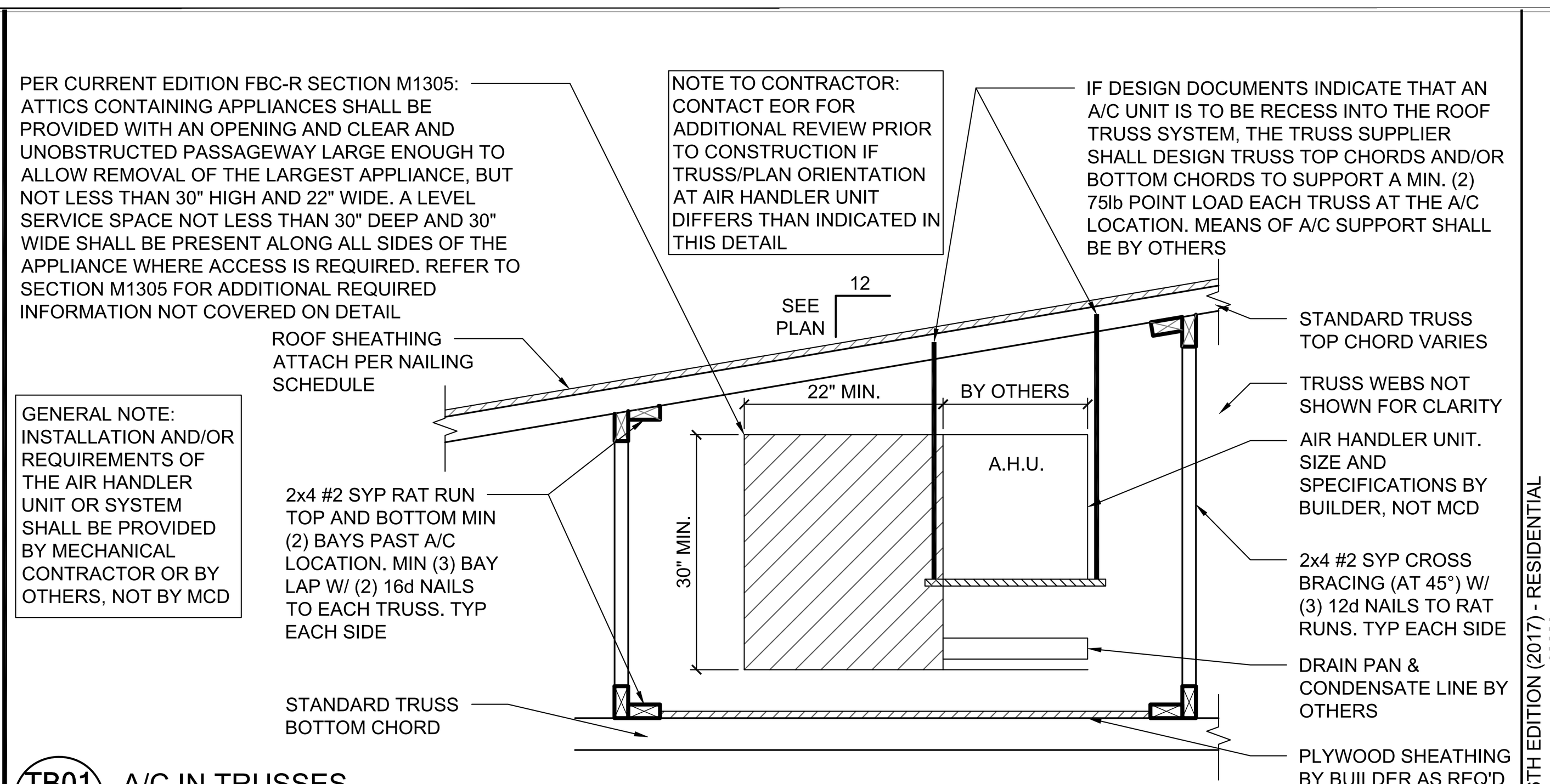


ST10 RECESSED STEEL COLUMN @ STEM WALL

3/4"=1'-0"

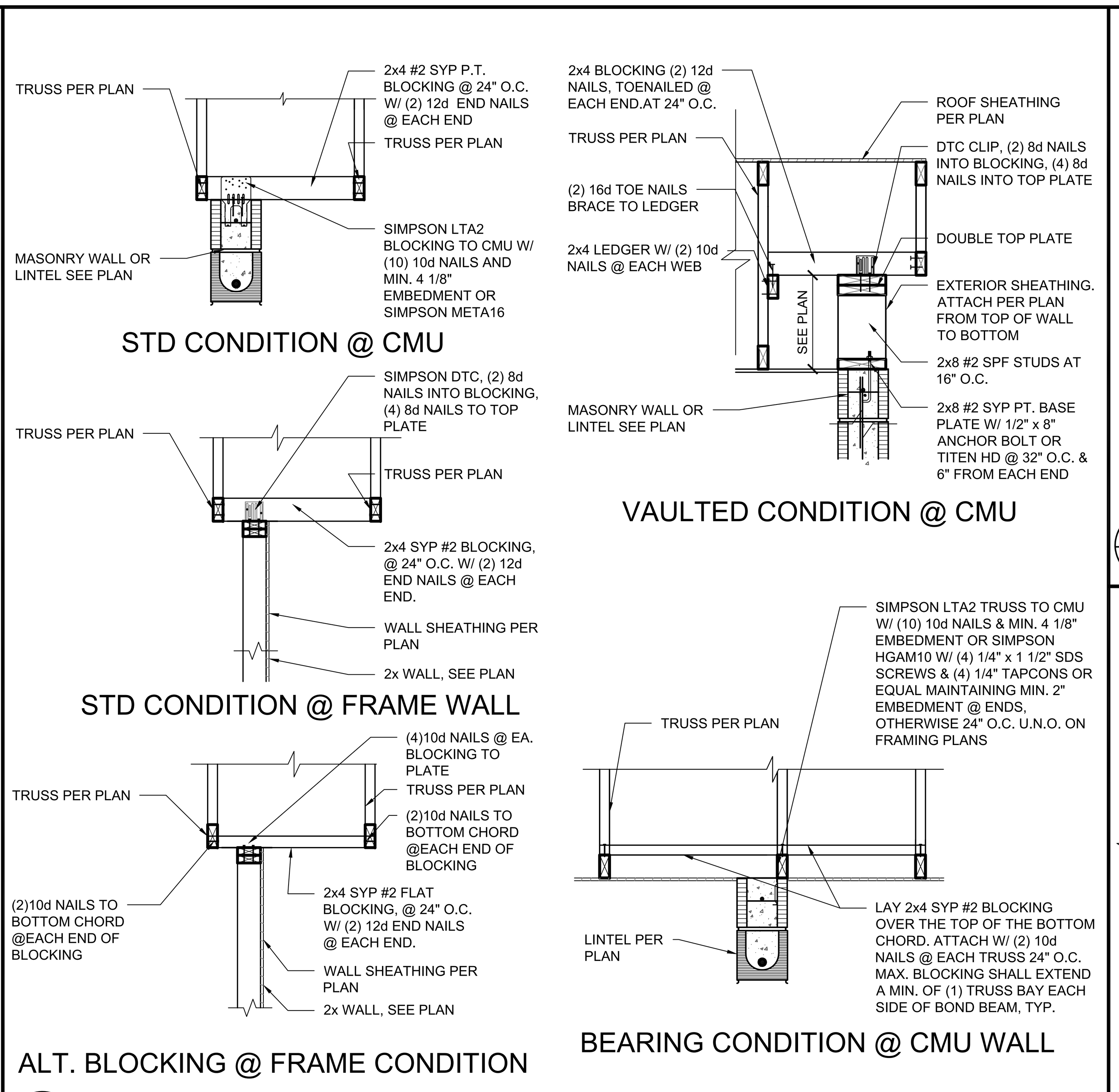


SECTION A-A



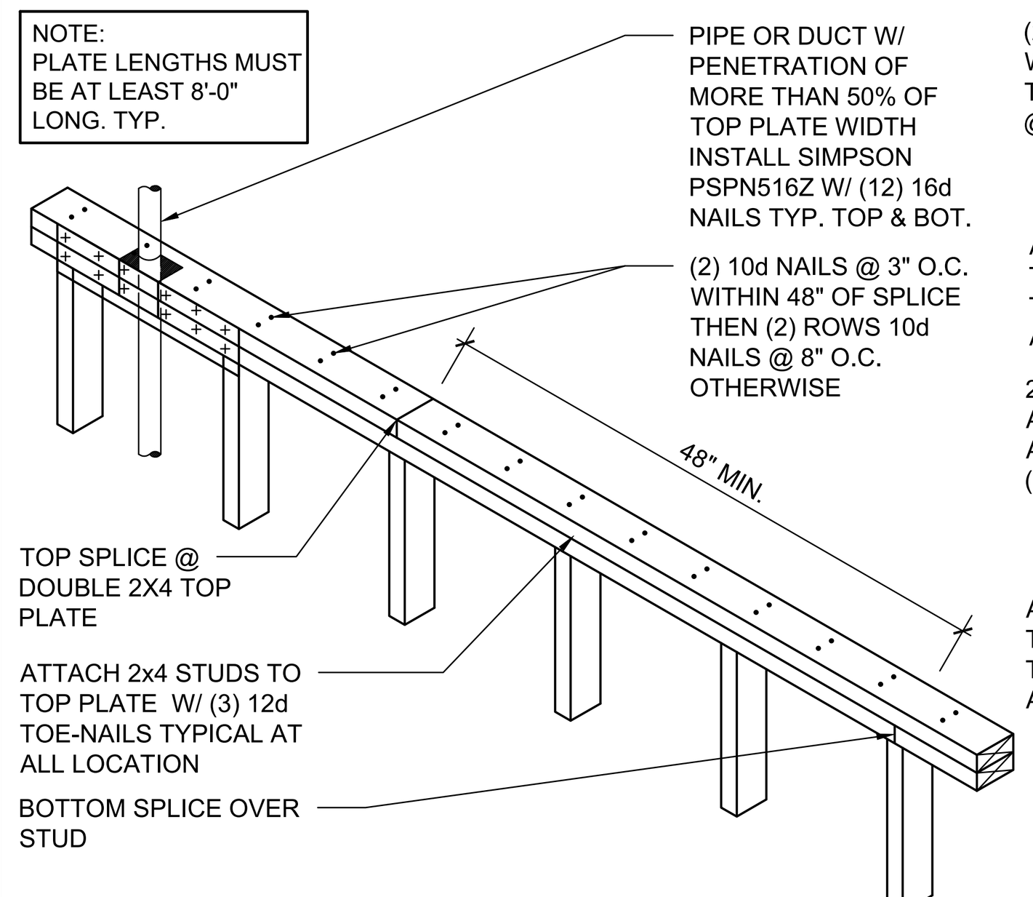
TB01 A/C IN TRUSSES

3/4"=1'-0"



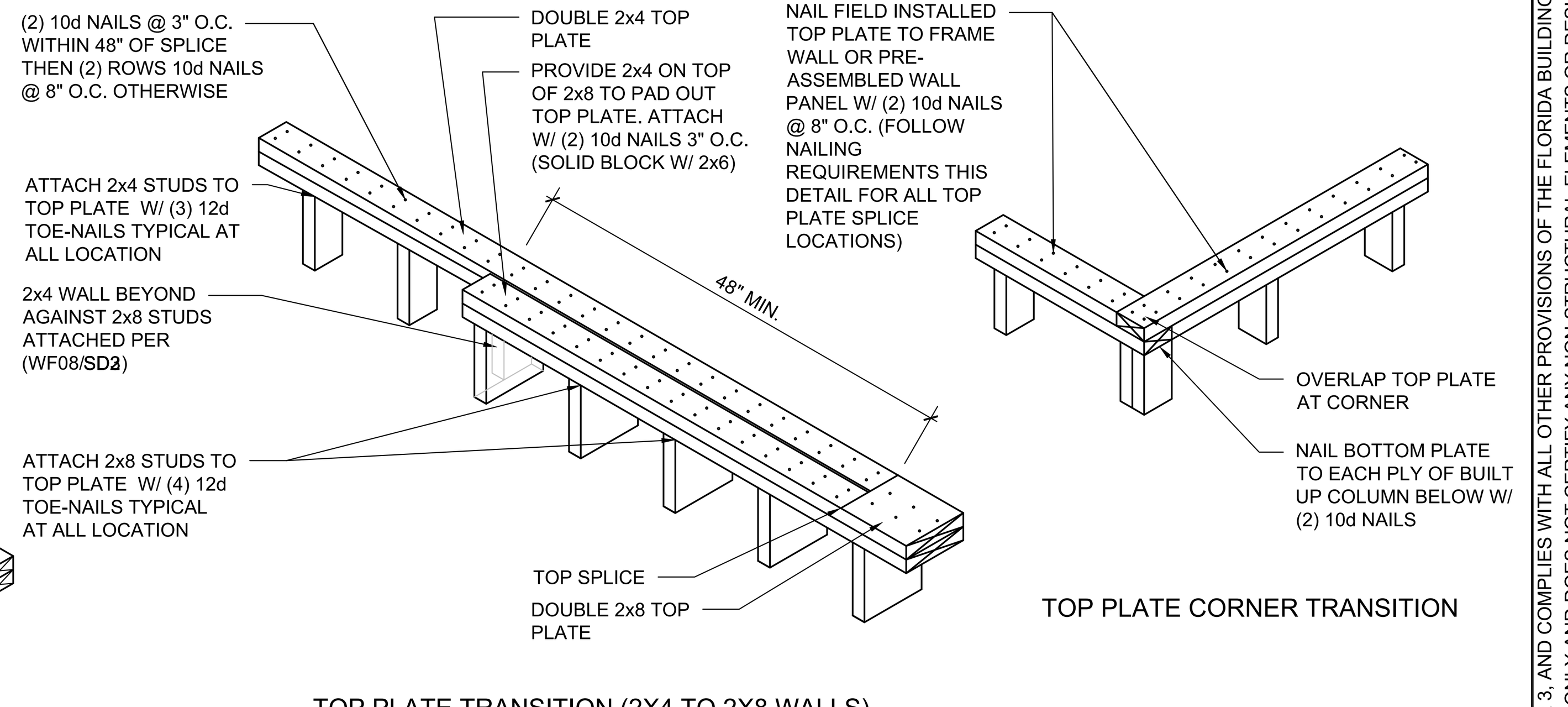
WF06 WALL CLIP DETAIL (150 MPH EXP. C MAX)

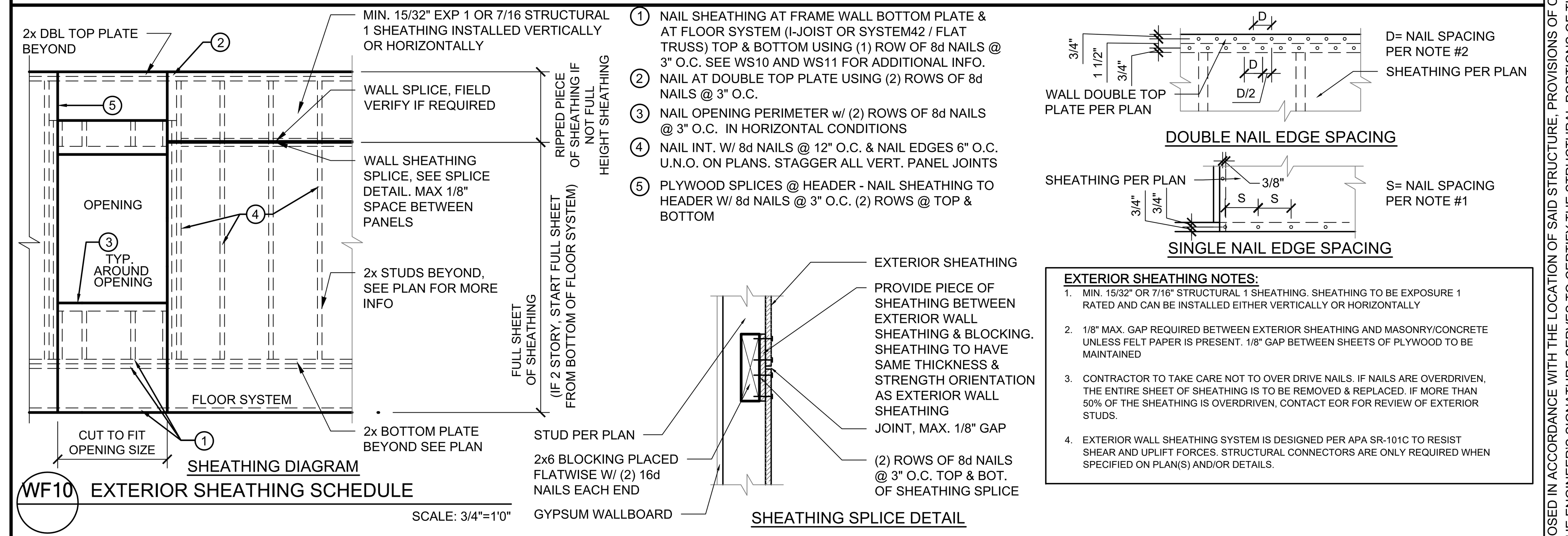
3/4"=1'-0"



WF07 TYPICAL TOP PLATE SPLICE @ BRG. WALL

3/4"=1'-0"





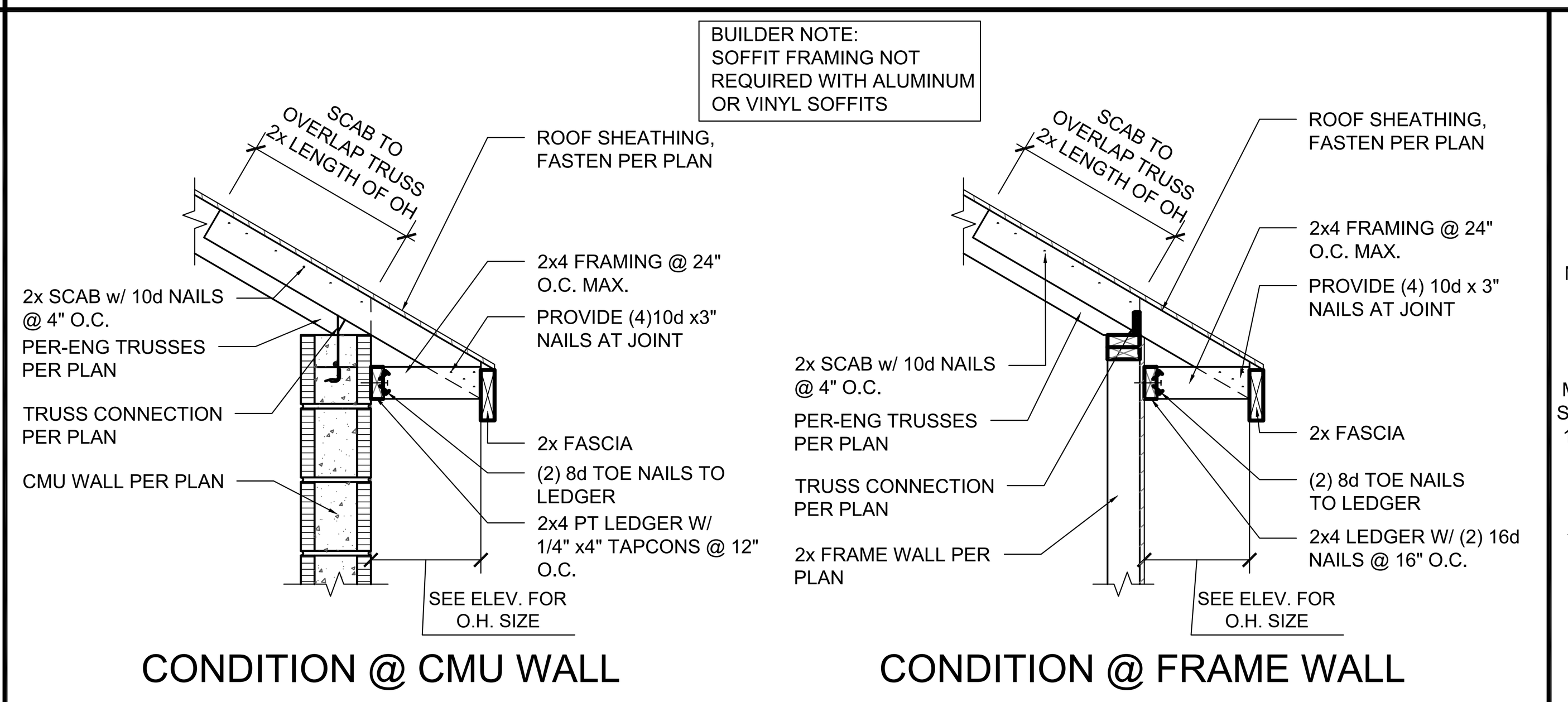
EXTERIOR SHEATHING NOTES:

1. MIN. 15/32" OR 7/16" STRUCTURAL 1 SHEATHING. SHEATHING TO BE EXPOSURE 1 RATED AND CAN BE INSTALLED EITHER VERTICALLY OR HORIZONTALLY
2. 1/8" MAX. GAP REQUIRED BETWEEN EXTERIOR SHEATHING AND MASONRY/CONCRETE UNLESS FELT PAPER IS PRESENT. 1/8" GAP BETWEEN SHEETS OF PLYWOOD TO BE MAINTAINED
3. CONTRACTOR TO TAKE CARE NOT TO OVER DRIVE NAILS. IF NAILS ARE OVERDRIVEN, THE ENTIRE SHEET OF SHEATHING IS TO BE REMOVED & REPLACED. IF MORE THAN 50% OF THE SHEATHING IS OVERDRIVEN, CONTACT EOR FOR REVIEW OF EXTERIOR STUDS.
4. EXTERIOR WALL SHEATHING SYSTEM IS DESIGNED PER APA SR-101C TO RESIST SHEAR AND UPLIFT FORCES. STRUCTURAL CONNECTORS ARE ONLY REQUIRED WHEN SPECIFIED ON PLAN(S) AND/OR DETAILS.

WF10 EXTERIOR SHEATHING SCHEDULE

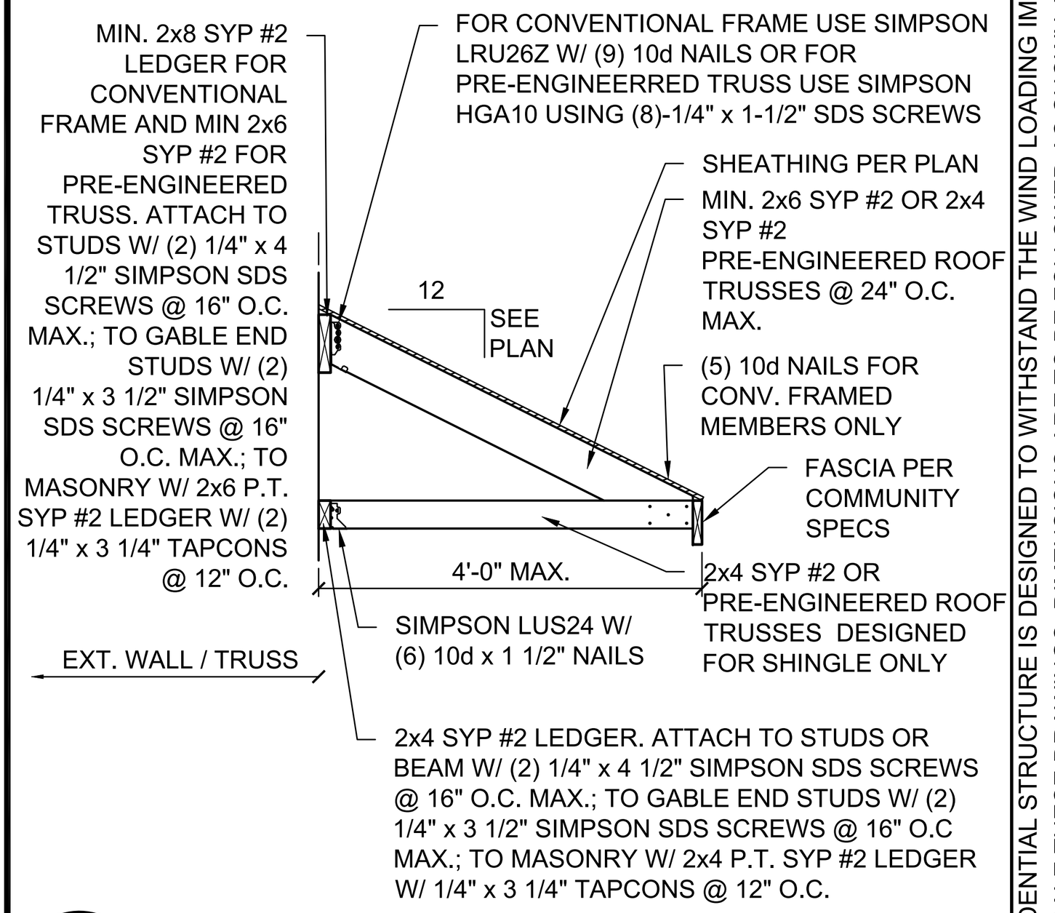
SCALE: 3/4"=1'0"

NOT USED



WF11 TYPICAL SCAB OVERHANG

3/4"=1'-0"



SD01 TYPICAL SHED ROOF DETAIL

1/2"=1'-0"

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COLIN F. GARDELLA, P.E.
FLORIDA LIC. No. 73199

REV.	DESCRIPTION	DATE

SHEET TITLE
GENERAL DETAILS

MONTA CONSULTING & DESIGN
OF WMR AND ASSOCIATES, LLC
CERT. OF AUTHORIZATION No. 32217
1035 GREENWOOD BLVD., SUITE 173
LAKE MARY, FL 32746
PHONE: (407) 681-1917
www.montaconsulting.com

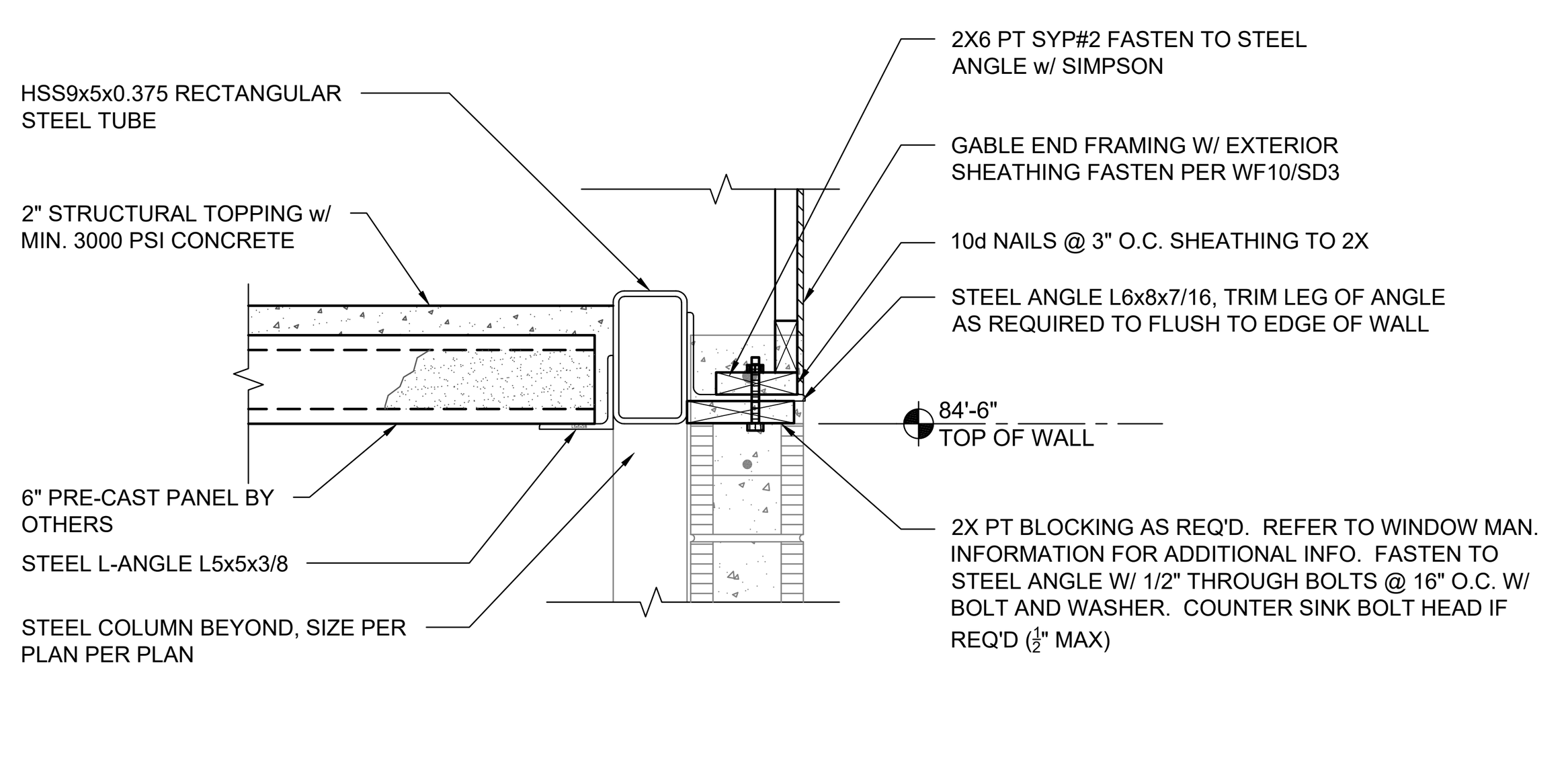
MCD STRENGTH BY DESIGN

PROJECT
COHEN RESIDENCE
1204 N. PARK AVE.
WINTER PARK, FL 32789

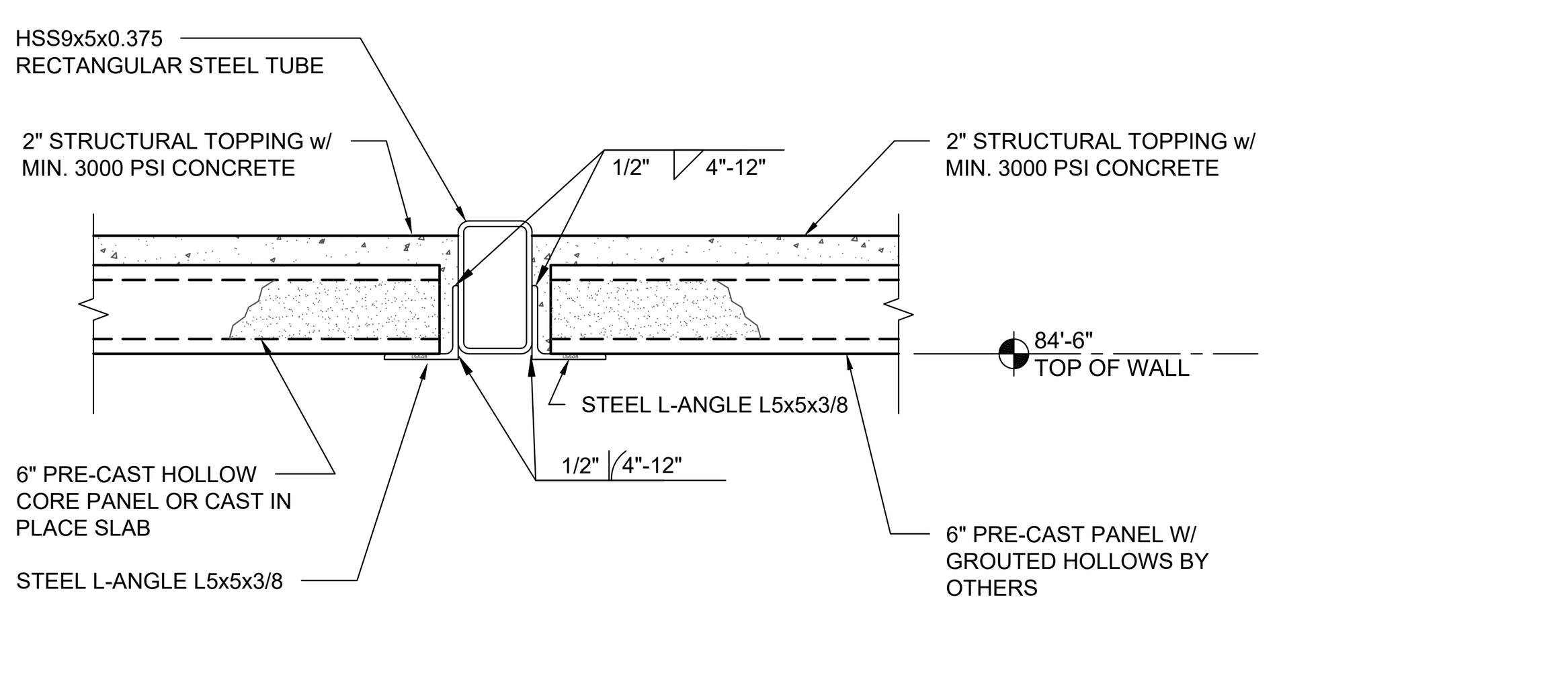
STRUCTURAL ENG:	***
DRAFTING:	
-STRUCT:	***
QC CHECK:	***
JOB No.	ZPROP-0206279
SCALE:	AS INDICATED
DATE:	8.26.19

SD3

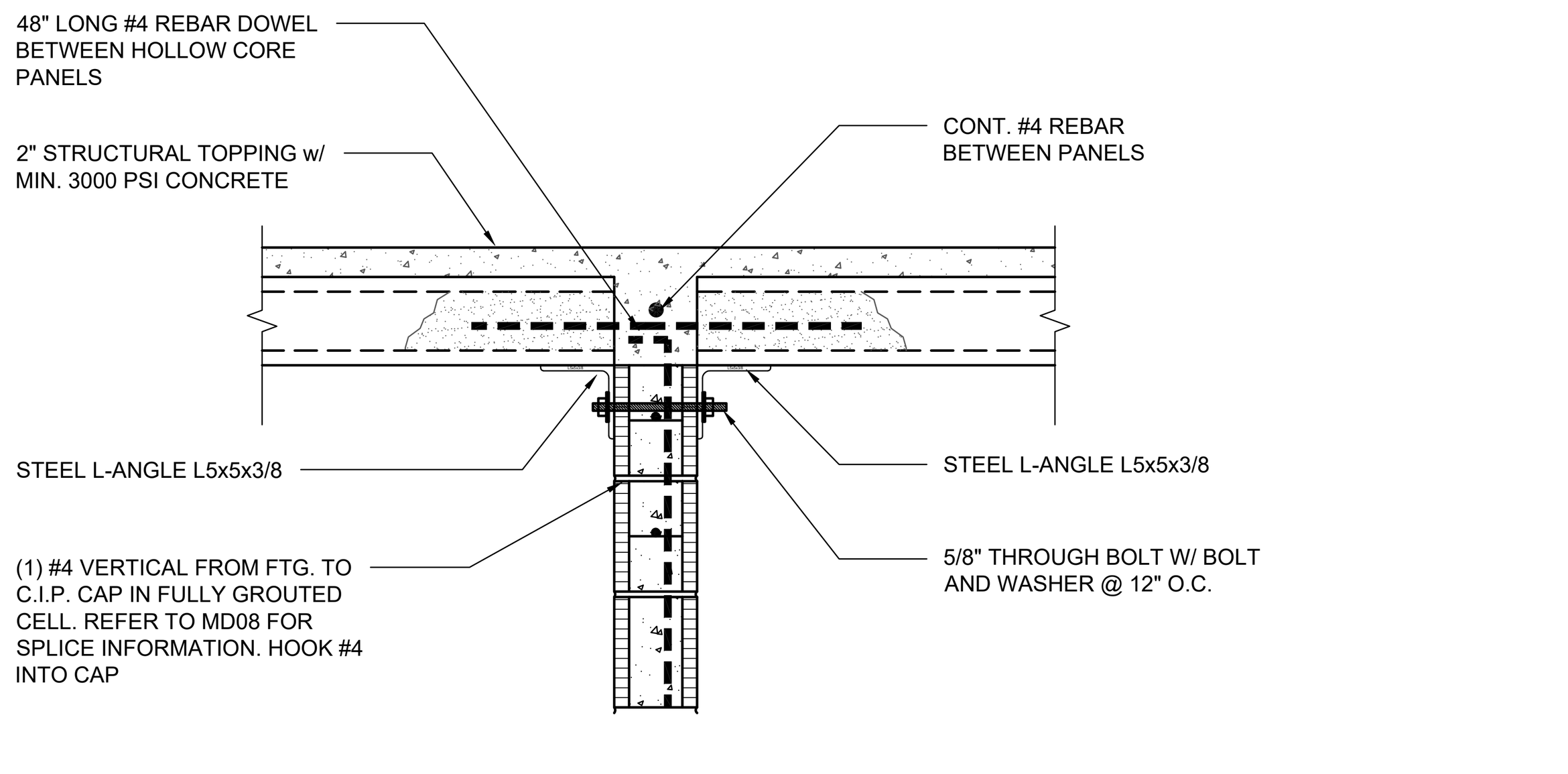
NOT USED



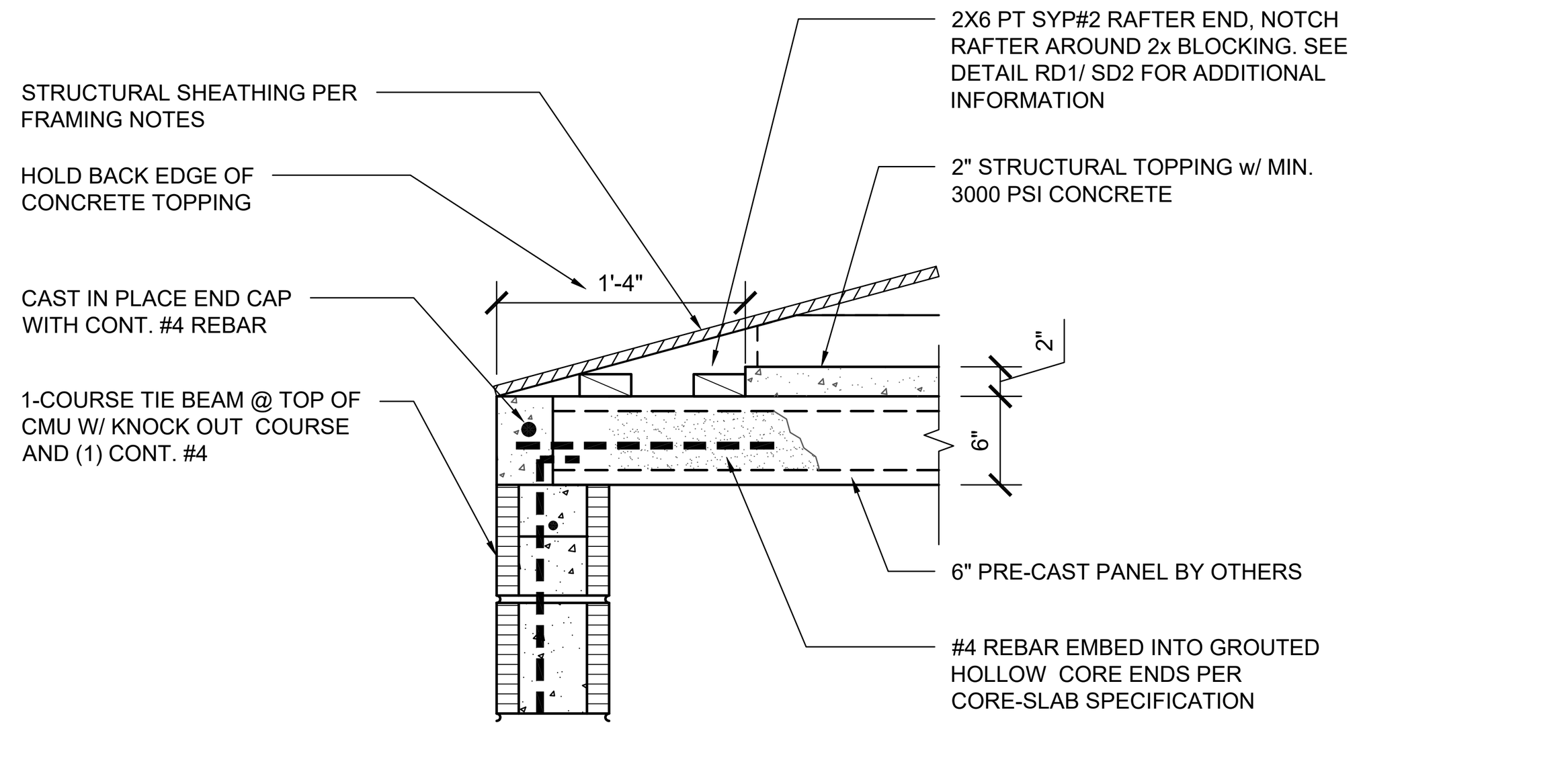
1 STEEL HEADER AT GALLERY - 1 SIDED
1"=1'-0"



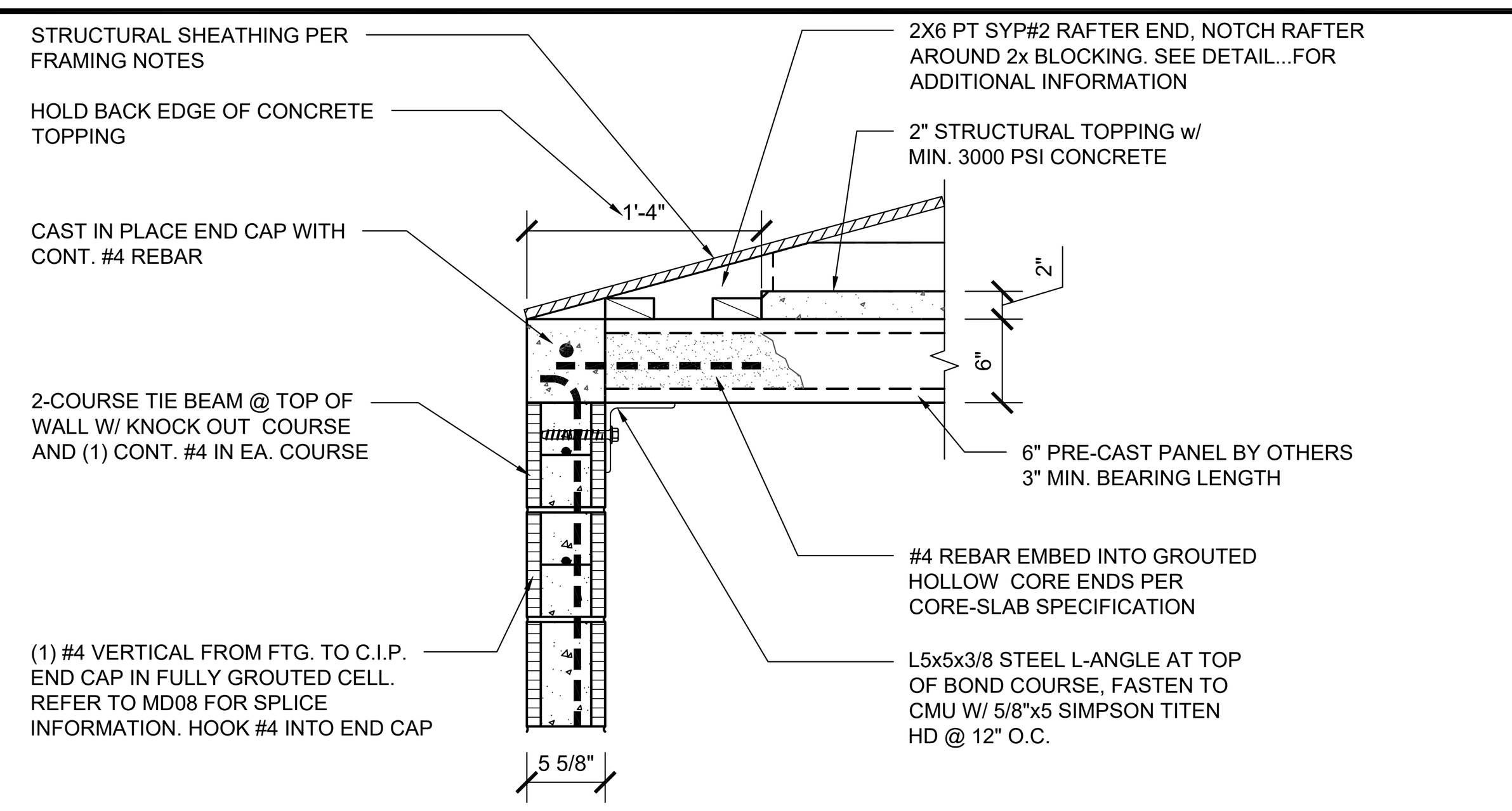
2 STEEL HEADER AT GALLERY - 2 SIDED
1"=1'-0"



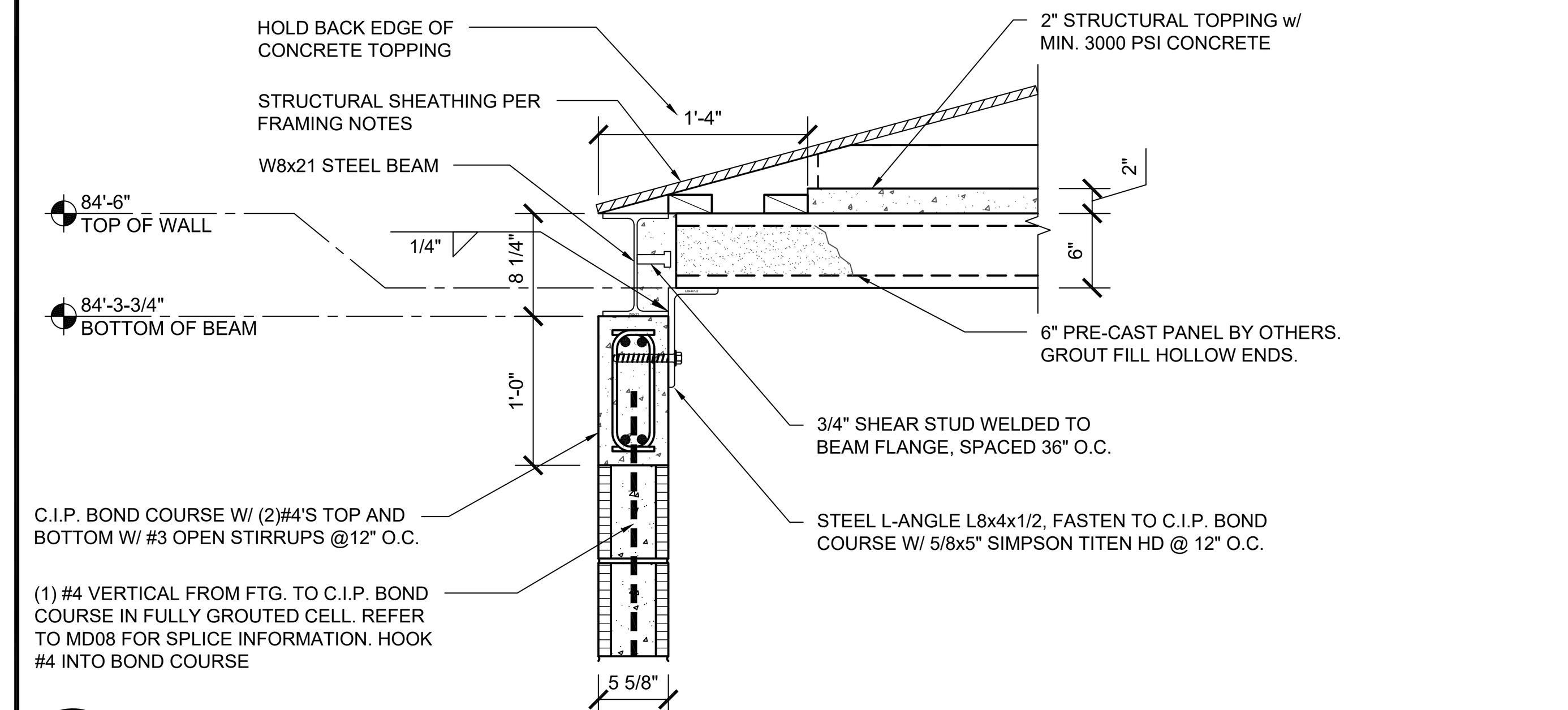
3 2- SIDED HOLLOW CORE BEARING @ 6" CMU
1"=1'-0"



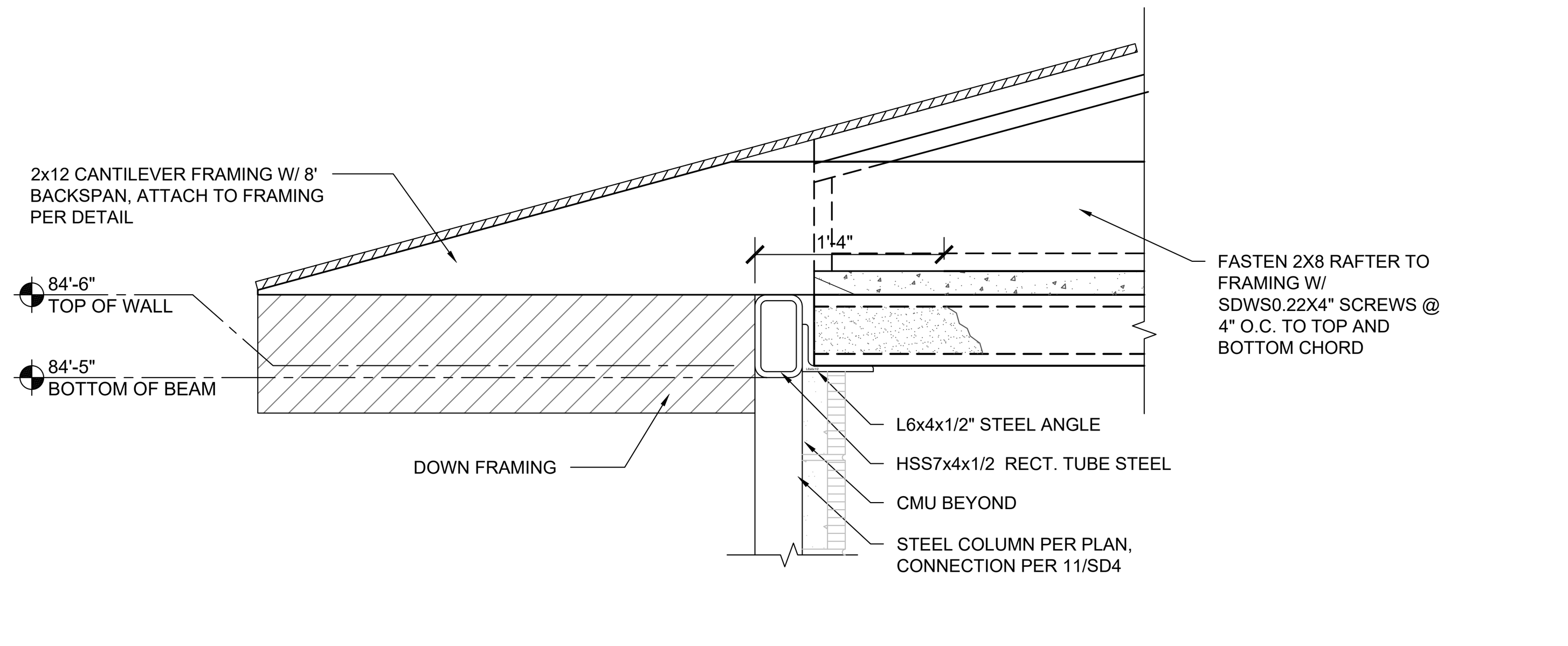
4 HOLLOW CORE BEARING @ 8" CMU
1"=1'-0"



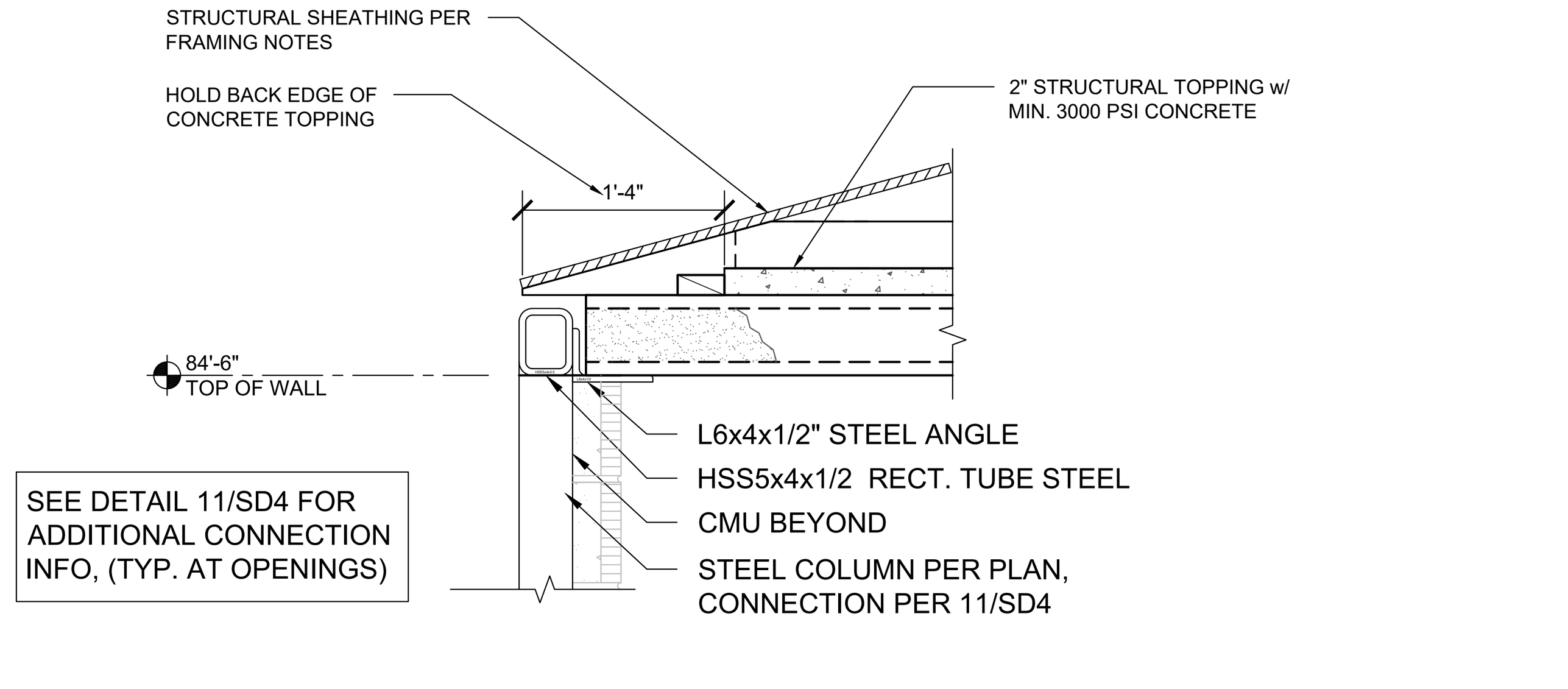
5 HOLLOW CORE BEARING @ 6" CMU
1"=1'-0"



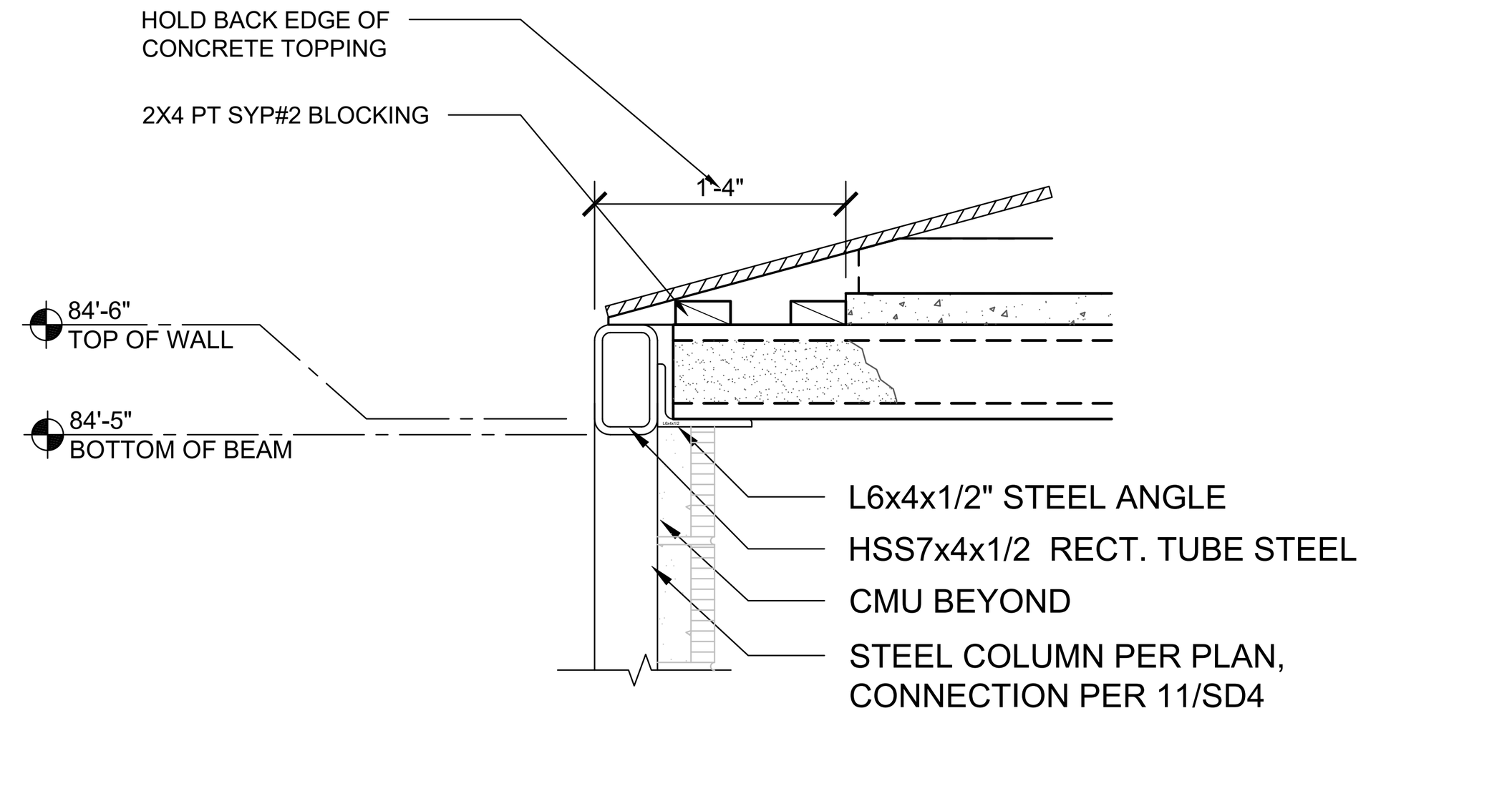
6 GARAGE CANTILEVER BEAM CONNECTION
1"=1'-0"



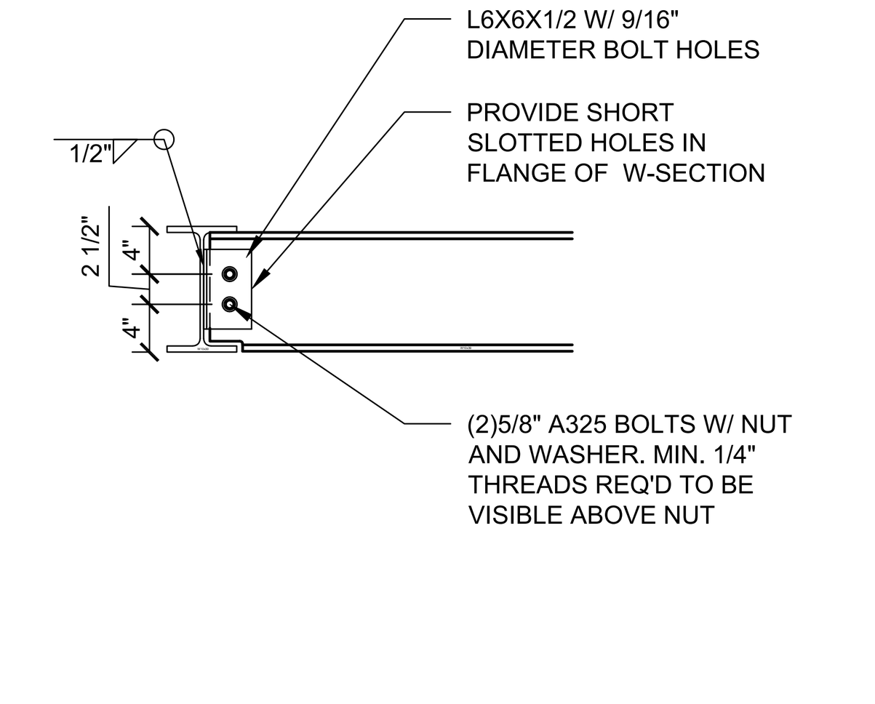
16 CANTILEVER FRAMING



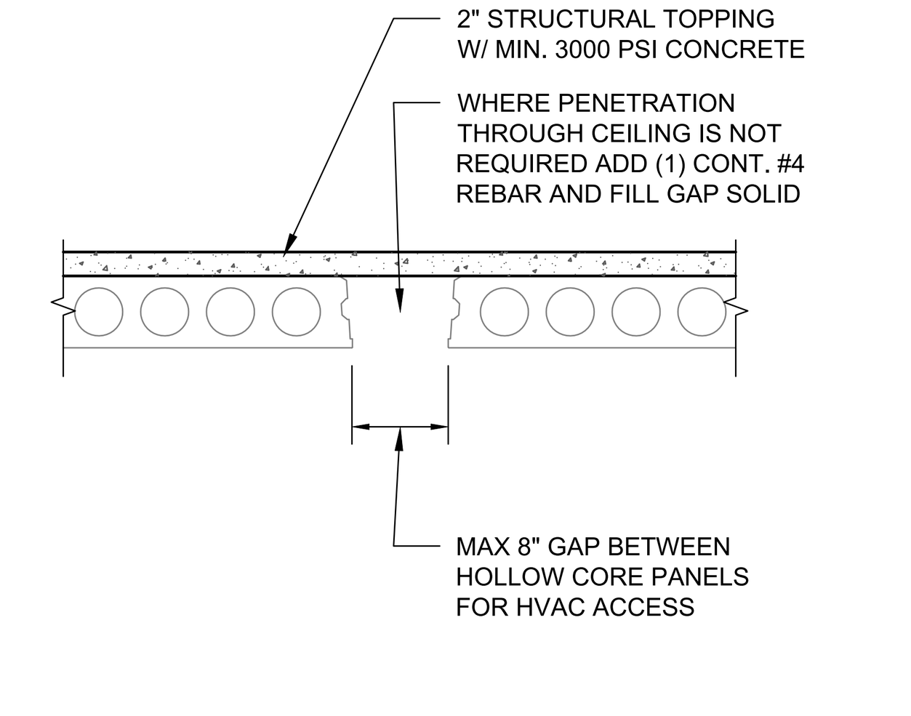
7 SHORT SPAN STEEL HEADER
1"=1'-0"



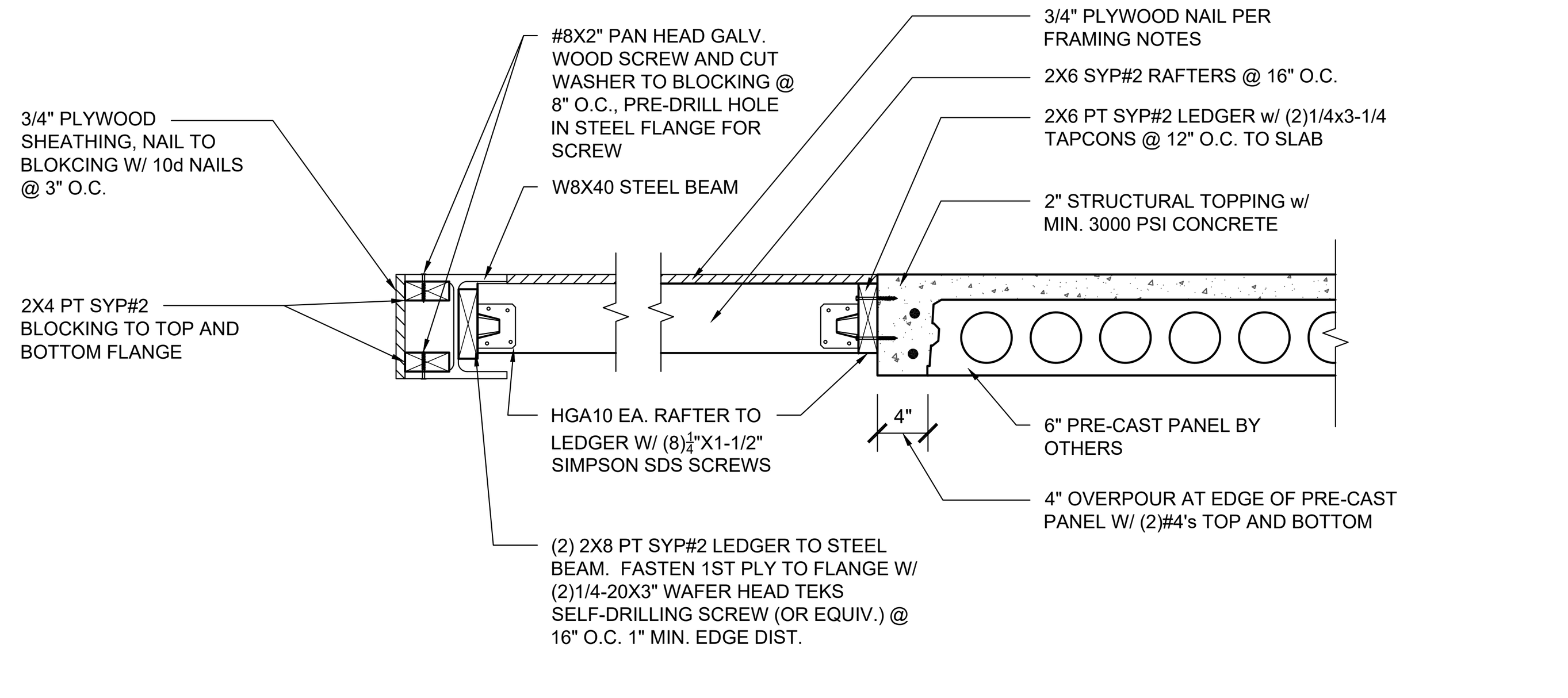
8 LONG SPAN STEEL HEADER
1"=1'-0"



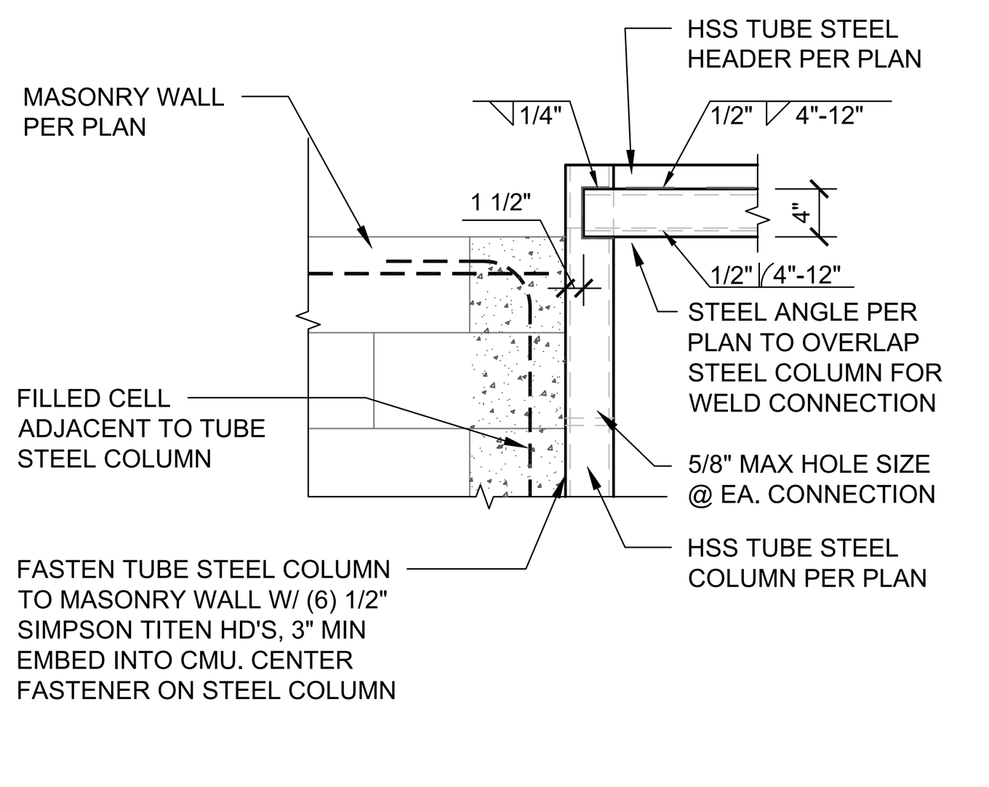
9 STEEL BEAM COPING AND CONNECTION DETAIL (GARAGE)
3/4"=1'-0"



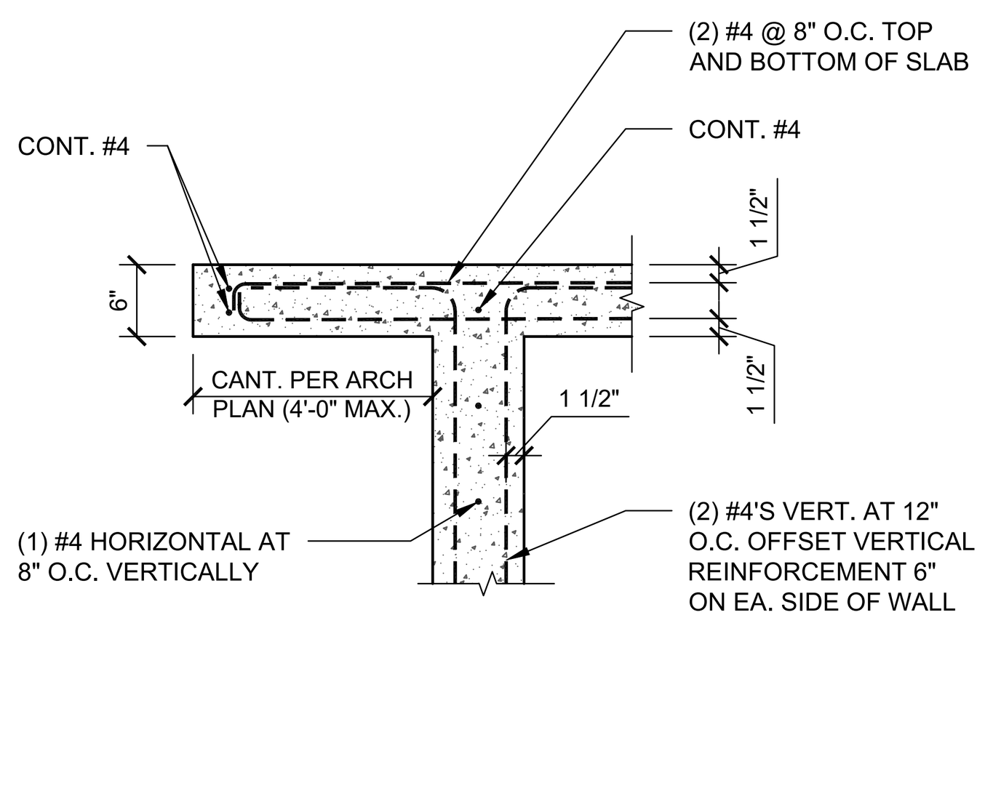
14 HOLLOW CORE PANEL GAP FOR UTILITY ACCESS
3/4"=1'-0"



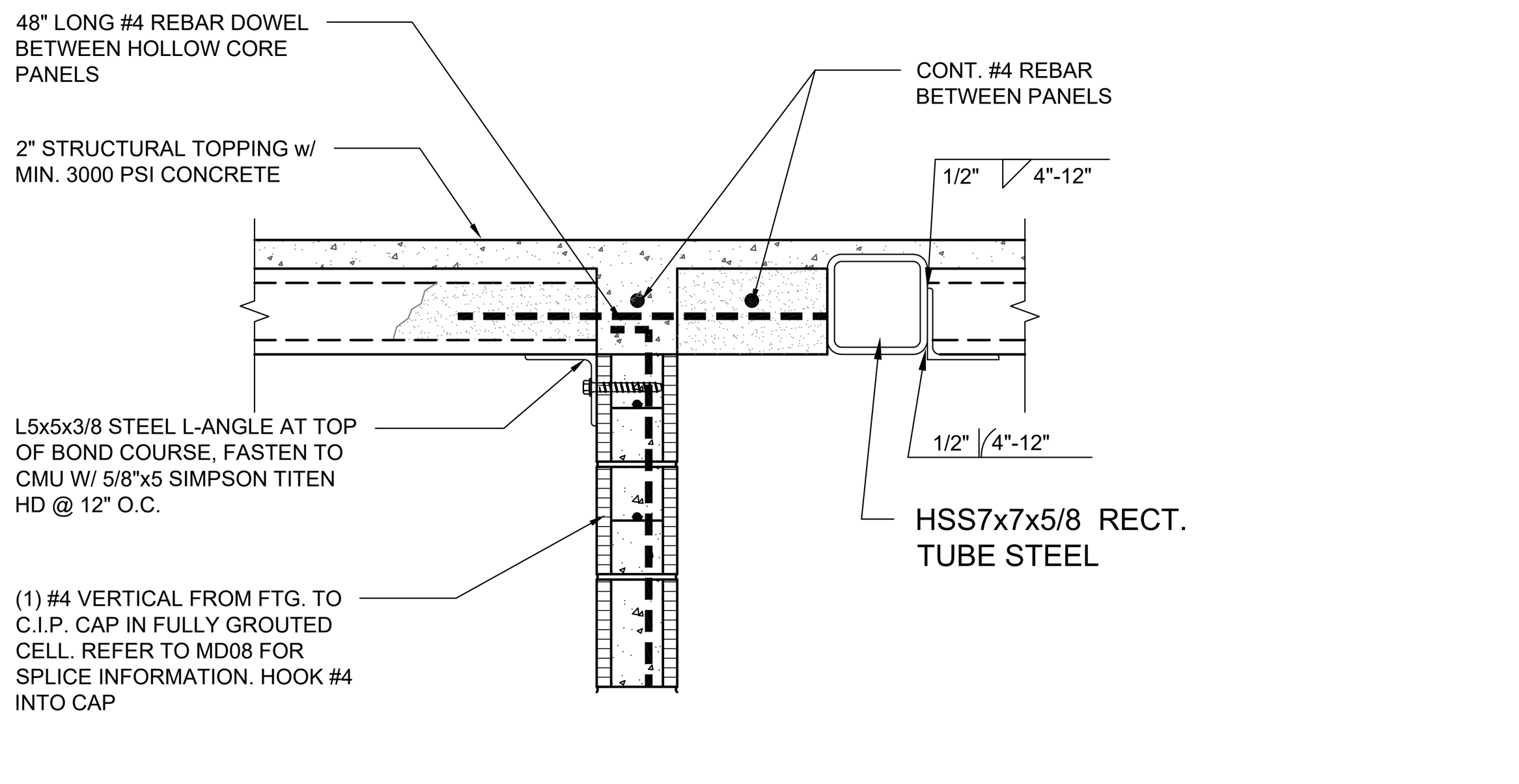
10 FRAMING ABOVE GARAGE DOOR
1"=1'-0"



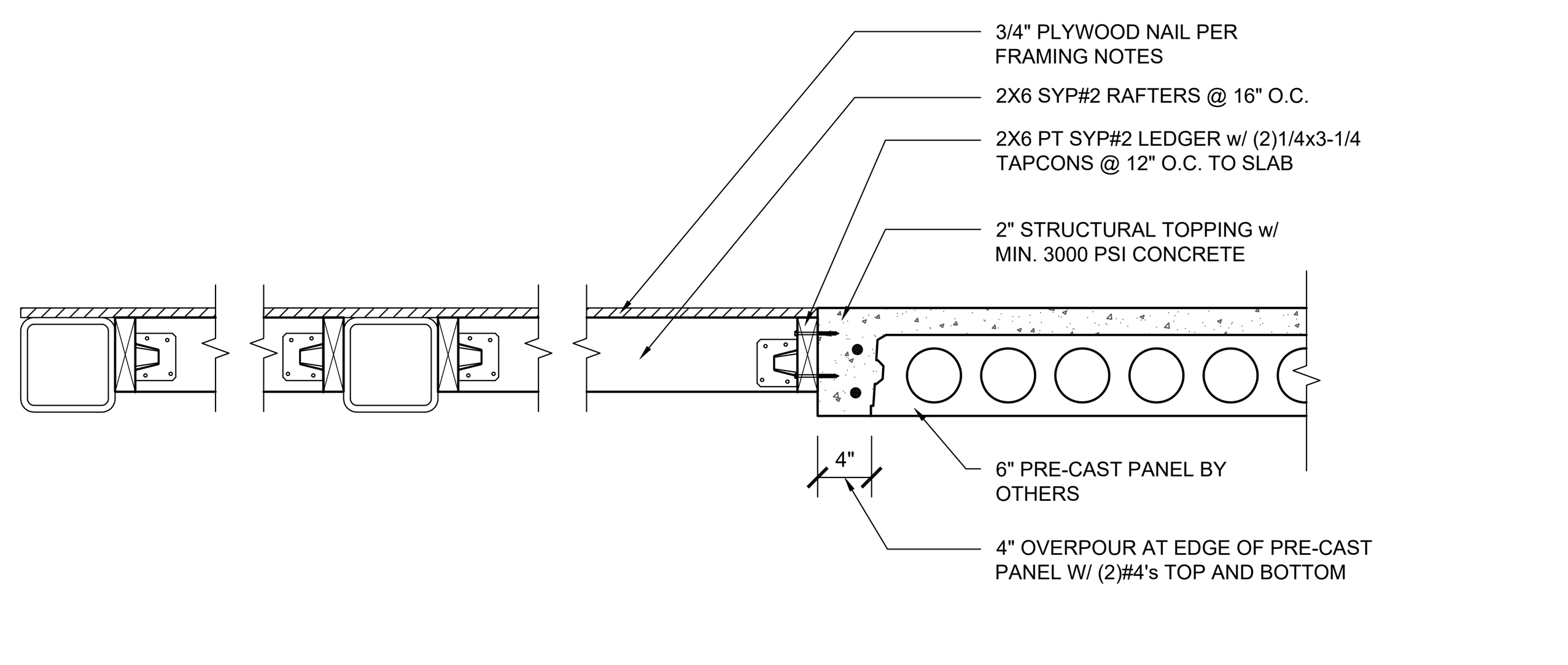
11 STEEL BEAM CONNECTION DETAIL
3/4"=1'-0"



12 CANTILEVER SLAB
3/4"=1'-0"



13 2- SIDED HOLLOW CORE BEARING @ 6" CMU
1"=1'-0"



15 FRAMING ABOVE BACK PORCH
1"=1'-0"

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COLIN F. GARDELLA LICENSE No. 73199 STATE OF FLORIDA PROFESSIONAL ENGINEER

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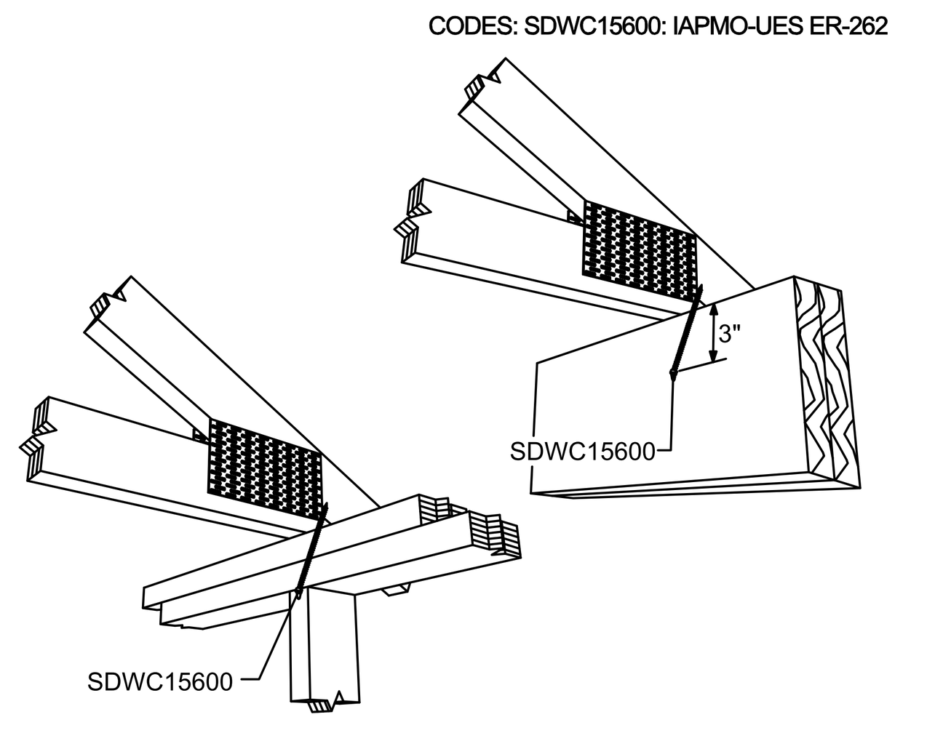
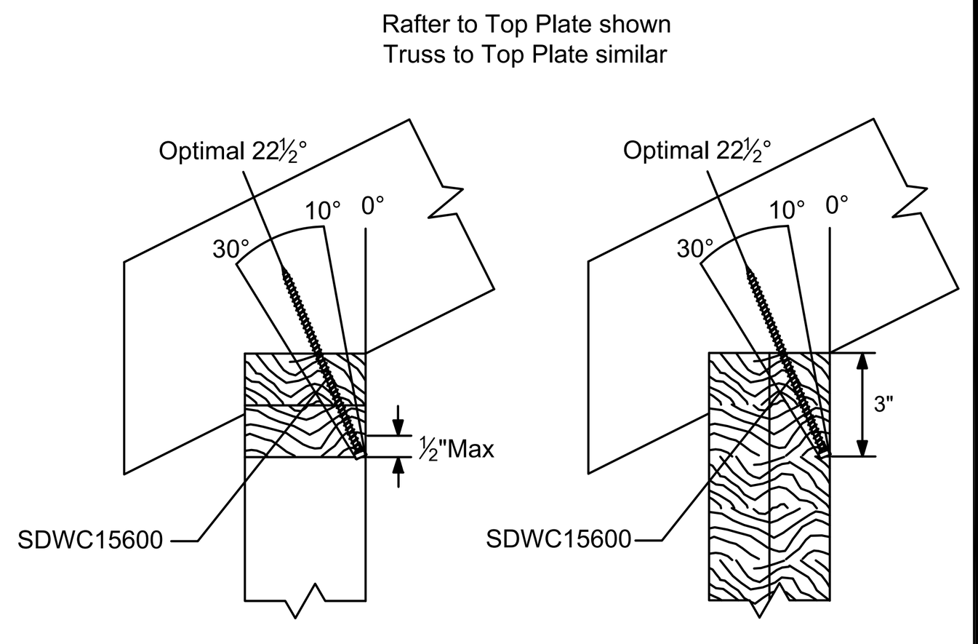
SD4



SINGLE SDWC ROOF TO WALL OR BEAM INSTALLTION

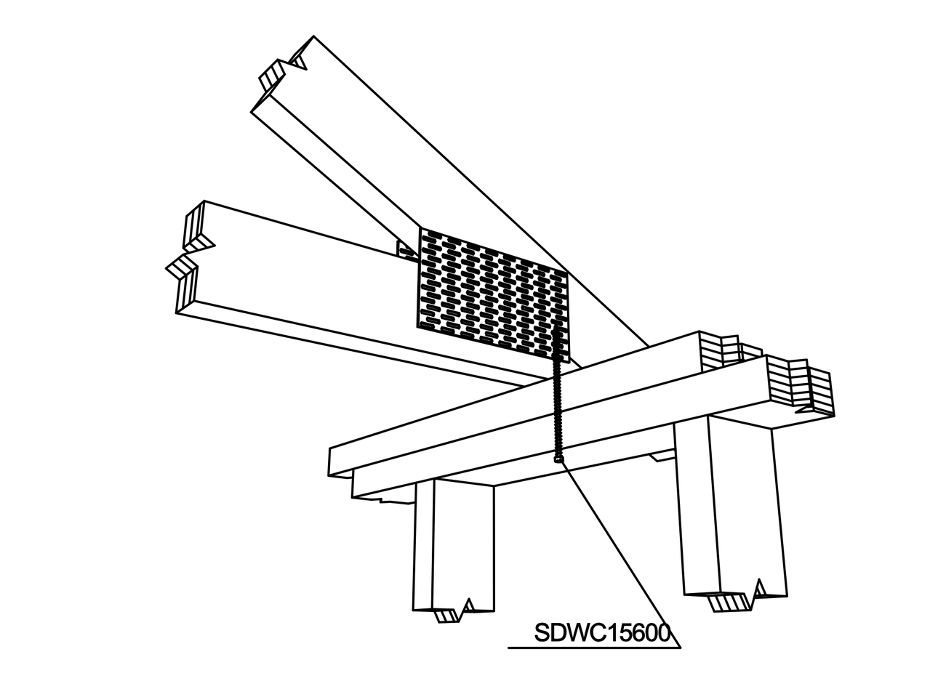
1 SINGLE SDWC ROOF TO WALL OR BEAM INSTALLATION
N.T.S.



Note: 1. Sloped-roof rafters may be sloped up to and including a 12:12 pitch and must be "birdsmouth" cut.
2. Reference detail 4 for installation instructions.

SINGLE SDWC ROOF TO WALL OR BEAM INSTALLATION RANGE

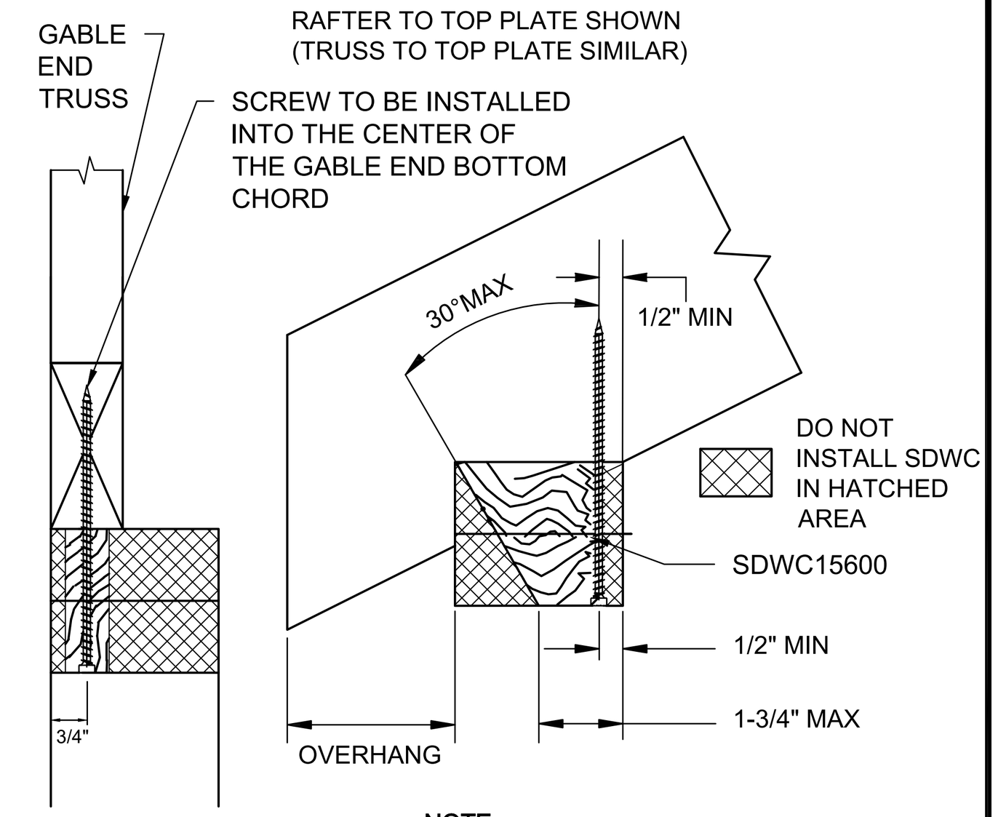
1a SINGLE SDWC ROOF TO WALL OR BEAM INSTALLATION RANGE
N.T.S.



Note: Reference detail 2a for installation angle limit

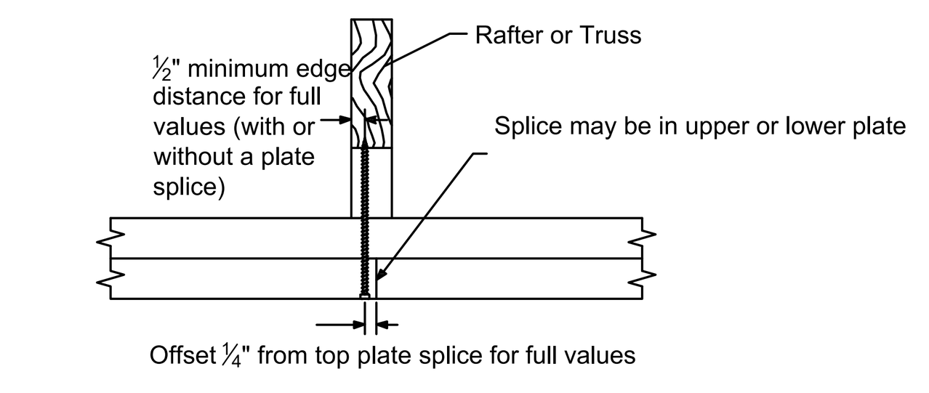
OPT. SDWC INSTALLATION - TRUSS/RAFTER OFFSET FROM STUD

2 OPT. SDWC INSTALLATION-TRUSS/RAFTER OFFSET FROM STUD
N.T.S.



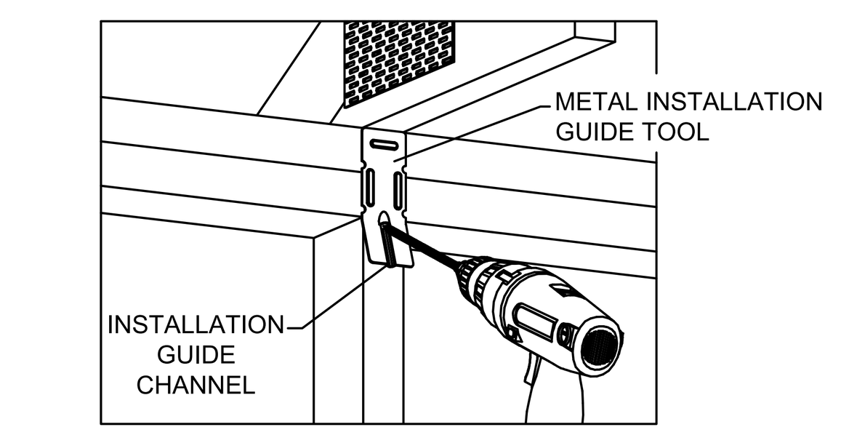
NOTE:
SLOPED-ROOF RAFTERS MAY BE SLOPED UP TO AND INCLUDING A 12:12 PITCH AND MUST BE "BIRDSMOUTH" CUT.

2a SDWC OPT. SDWC INSTALLATION RANGE
3"=1'-0"



SDWC MIN. EDGE DISTANCE FOR TOP PLATE SPLICE

3 SDWC MIN. EDGE DISTANCE FOR TOP PLATE SPLICE
N.T.S.



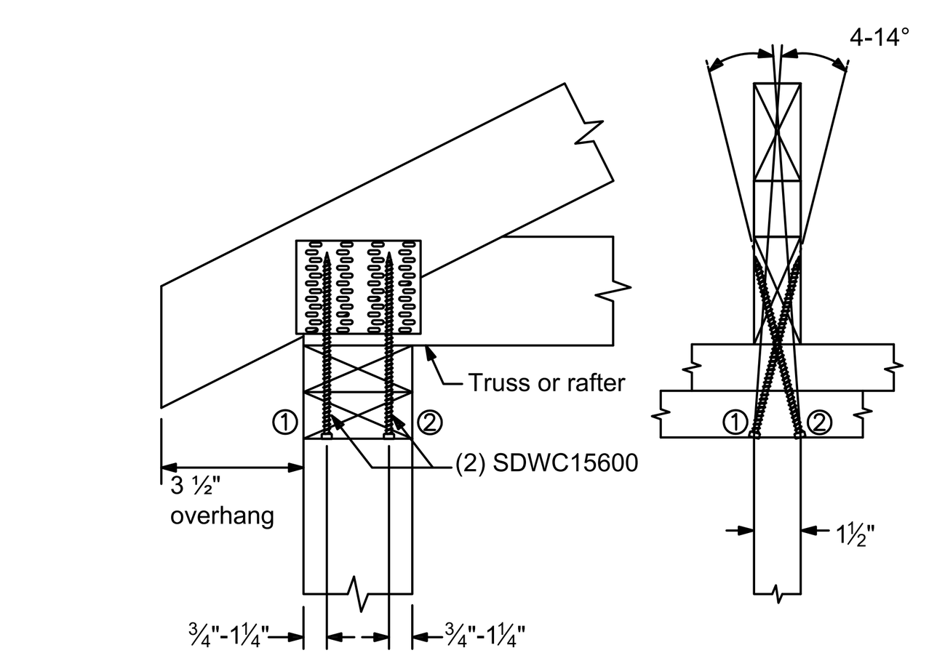
Step 1 - Align the metal installation guide tool (included) with the truss or rafter, and drive the tip of the Strong-Drive SDWC to engage the threads.

Step 2 - While continuing to drive the SDWC, "drop" the fastener head into the guide channel to ensure optimal installation angle of 22.5°. The installation angle range is 10°-30° (see detail 1a). Once the installation angle is established, the metal installation guide tool may be removed.

Step 3 - Drive the SDWC until the head of the fastener is fully countersunk into the double top plate. Verify that the entire shank of the fastener is installed into a wood member.

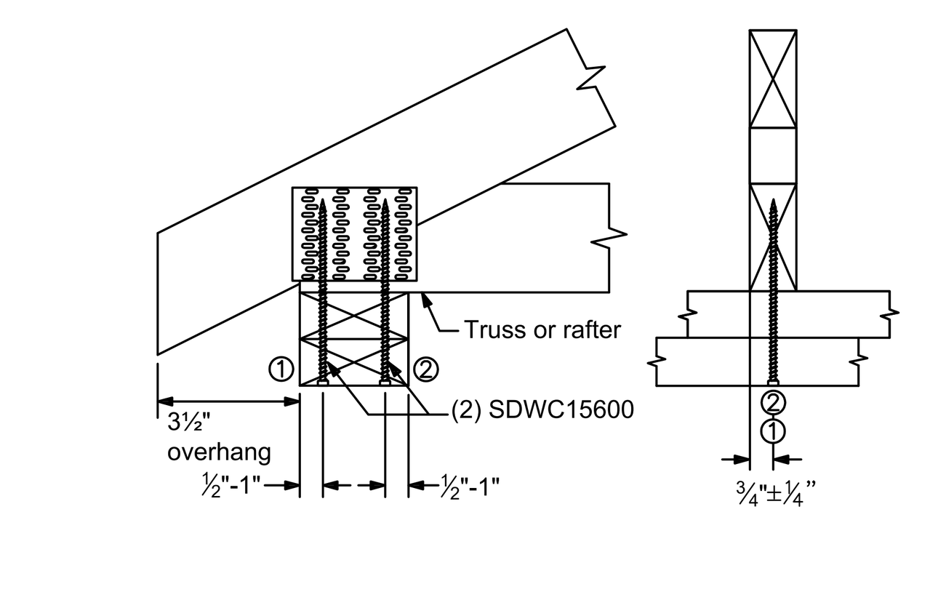
SDWC INSTALLATION INSTRUCTIONS (ROOF TO WALL)

4 SDWC ISTALLATION INSTRUCTIONS (ROOF TO WALL)
N.T.S.



DOUBLE SDWC INSTALL: CONFIGURATION A

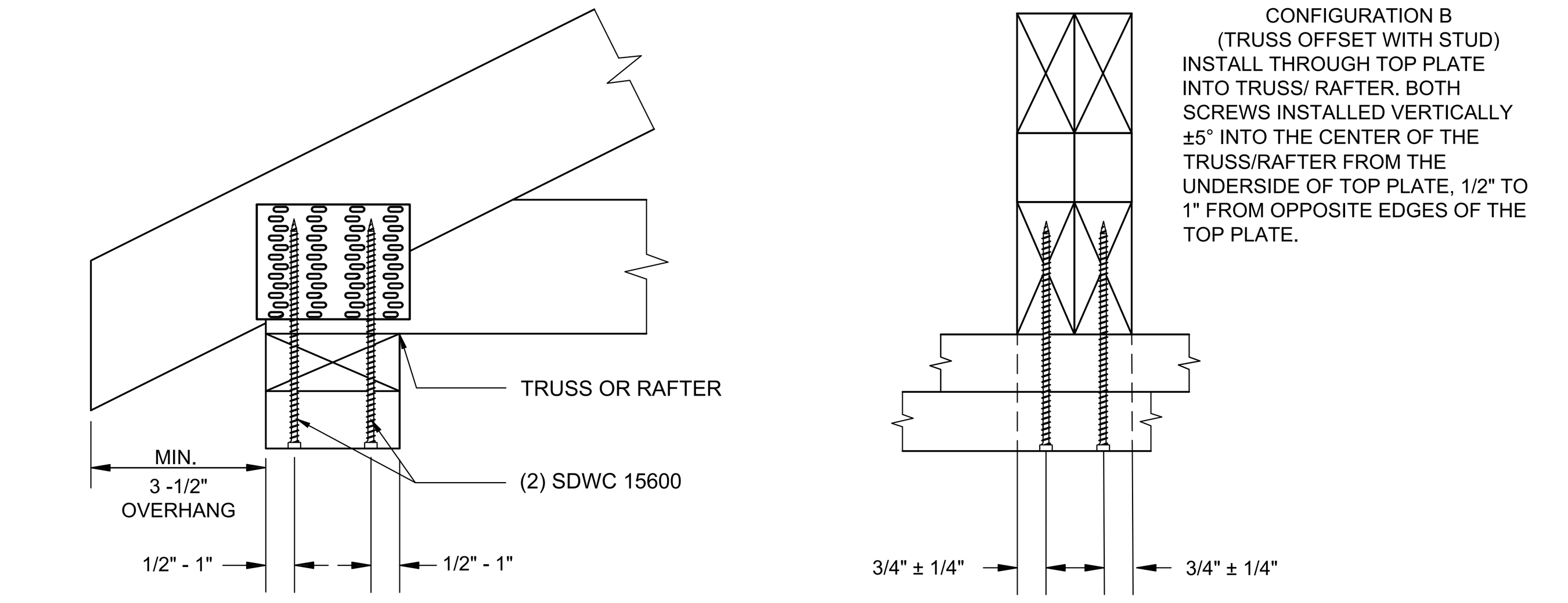
5 DOUBLE SDWC INSTALL: CONFIGURATION A
N.T.S.



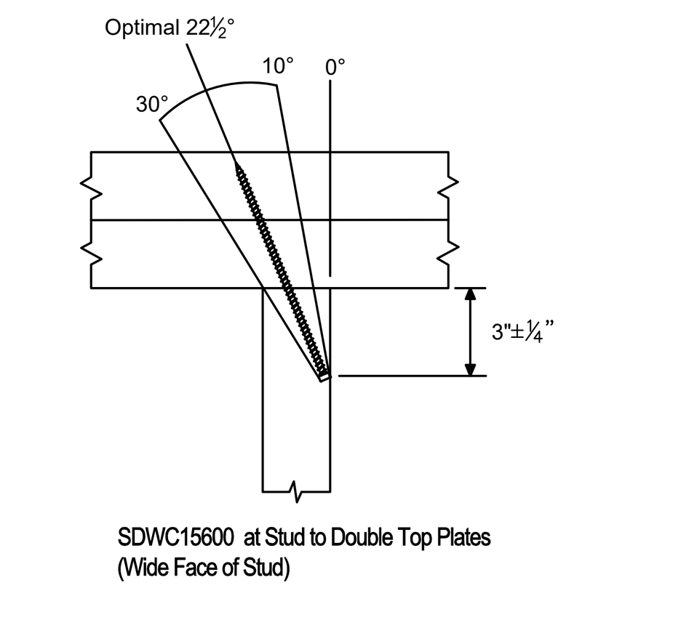
Both screws installed ± 5°.

DOUBLE SDWC INSTALL: CONFIGURATION B

6 DOUBLE SDWC INSTALL: CONFIGURATION B
N.T.S.



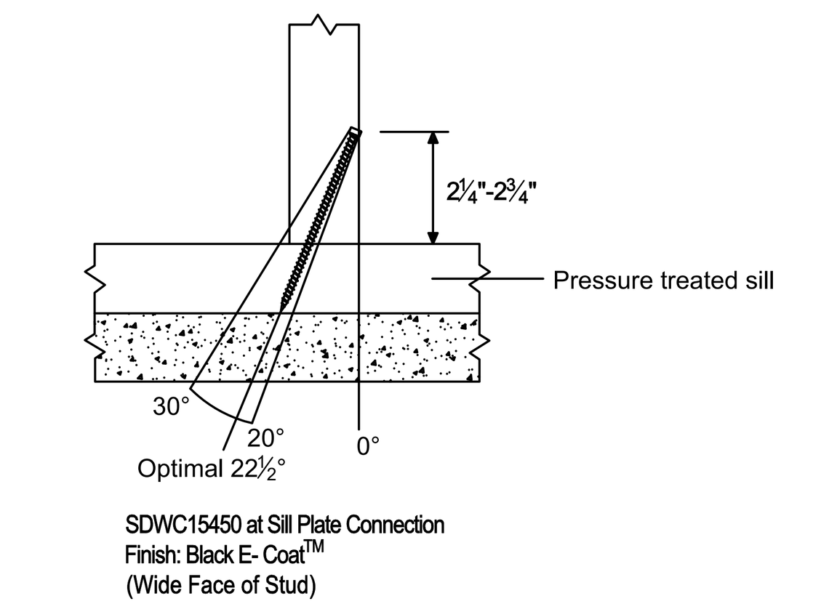
6a OPT. SDWC INSTALLATION-TRUSS/RAFTER OFFSET FROM STUD
N.T.S.



Note: Stud-to-double top plates shown. Stud-to-single/double bottom plates over wood floor similar.

SDWC STUD-TO-TOP/BOTTOM PLATES CONNECTION

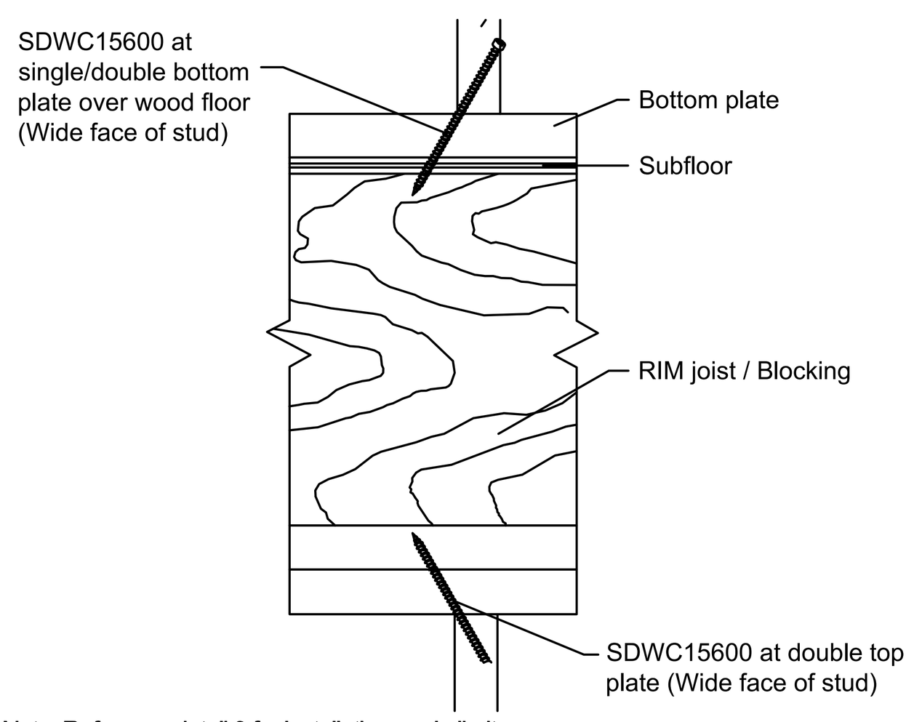
9 SDWC STUD-TO-TOP/BOTTOM PLATES CONNECTION
N.T.S.



Note: Sill plate anchor to foundation not shown for clarity.

SDWC STUD-TO-SILL PLATE CONNECTION

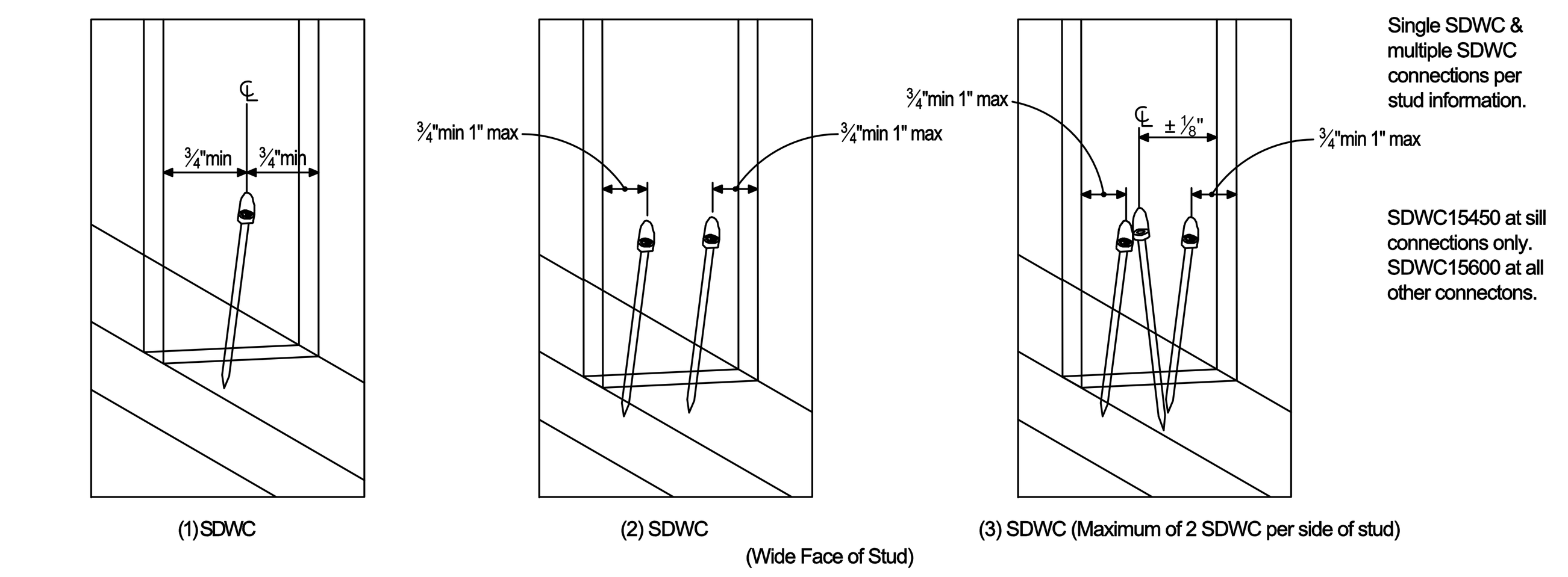
10 SDWC STUD-TO-SILL PLATE CONNECTION
N.T.S.



Note: Reference detail 9 for installation angle limit

SDWC STUD-TO-BOTT. PLATE CONNECTION OVER WOOD FLOOR

11 SDWC STUD-TO-BOTT. PLATE CONNECTION OVER WOOD FLOOR
N.T.S.



Single SDWC & multiple SDWC connections per stud information.

SDWC15450 at sill connections only. SDWC15600 at all other connectons.

Note: Stud-to-Bottom Plate shown. All other installations similar.

SDWC EDGE DISTANCE AND SPACING INFORMATION

12 SDWC EDGE DISTANCE AND SPACING INFORMATION
N.T.S.

NOTES

1. STRONG-DRIVE STRUCTURAL WOOD SCREWS FOR TRUSS/RAFTER, STUD-TO-PLATE, AND FLOOR-TO-FLOOR CONNECTIONS ARE MANUFACTURED AND TRADEMARKED BY "SIMPSON STRONG-TIE COMPANY, INC." HOME OFFICE: 5956 W. LAS POSITAS BLVD., PLEASANTON, CA 94588 TEL: (800) 999-5099, FAX: (925) 847-1597. "SIMPSON STRONG-TIE COMPANY, INC." IS AN ISO 9001 REGISTERED COMPANY.
2. USE OF THIS PRODUCT IS SUBJECT TO THE APPROVAL OF THE LOCAL BUILDING DEPARTMENT.
3. THESE PRODUCTS ARE PART OF THE OVERALL WIND UPLIFT FORCE RESISTING SYSTEM OF THE STRUCTURE. DESIGN OF THE BUILDING'S MAIN WIND FORCE RESISTING SYSTEM, INCLUDING THE LOAD PATH TO TRANSFER UPLIFT FORCES FROM THE STRUCTURE TO THE GROUND, IS THE RESPONSIBILITY OF THE SPECIFIER.
4. ENGINEER OF RECORD IS PERMITTED TO MODIFY DETAILS FOR SPECIFIC CONDITIONS.
5. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, ETC. PRIOR TO INSTALLATION OF ANY STRONG-DRIVE SCREWS FOR THE WIND UPLIFT RESISTING SYSTEM. IF ANY DISCREPANCIES ARE FOUND, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE SPECIFIER FOR CLARIFICATION PRIOR TO CONSTRUCTION. ONLY COMMON HEEL TRUSSES W/ A MIN. 3-1/2" OVERHANG CAN USE THESE SCREWS. RAISED HEEL/TAIL BEARING, ETC. MUST USE COMMON, METAL TRUSS CONNECTORS PER CONNECTER SCHEDULE.
6. INSTALLATION OF PRODUCT SHALL BE DONE IN CONFORMANCE TO THESE DRAWINGS. THE PERFORMANCE OF MODIFIED PRODUCTS OR ALTERED INSTALLATION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE FRAMING/GENERAL CONTRACTOR. CONTACT EOR IF INSTALLATION INSTRUCTIONS ARE NOT FOLLOWED.
7. SIMPSON STRONG-TIE COMPANY, INC. RESERVES THE RIGHT TO CHANGE SPECIFICATIONS, DESIGNS, AND MODELS WITHOUT NOTICE OR LIABILITY FOR SUCH CHANGES.
8. ALL HARDWARE CALLED OUT IS SIMPSON STRONG-TIE.
9. DOUBLE TOP PLATES SHALL BE FASTENED PER DETAIL WF07/ SD4
10. CONTRACTOR MAY USE SIMPSON SDWC15600 SCREWS ONLY FOLLOWING CONFIGURATION (A) OR (B) THIS SHEET IN LIEU OF SIMPSON H10A OR SIMPSON H2.5 FOR COMMON OR JACK TRUSSES WHERE UPLIFT DOES NOT EXCEED (1195 lbs) FOR DF/SP OR (1045 lbs) FOR SPF/HF.

- NOT USED
N.T.S.

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SDWC DETAILS

N · C · B · D · C
AIBD

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SD6